

GENERAL NOTES

1. PRIOR TO BEGINNING ANY WORK, THE CONTRACTOR AND SUB-CONTRACTORS SHALL INSPECT THE SITE TO BECOME FAMILIAR WITH ALL THE WORK AND VERIFY WITH THE ARCHITECT ANY ITEMS THAT ARE NOT FULLY UNDERSTOOD.
2. DURING DEMOLITION, THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO PROTECT FROM DAMAGE ALL WORK THAT IS TO REMAIN.
3. THE JOB SITE SHALL BE KEPT IN A SAFE CONDITION AND CLEAR OF ALL DEBRIS. ALL DEBRIS SHALL BE PLACED IN A DUMPSTER AT THE END OF EACH WORK DAY AND THE DUMPSTER SHALL BE EMPTIED WHEN FULL.
4. DURING CONSTRUCTION SHOULD THE CONTRACTOR UNCOVER ANY STRUCTURAL PROBLEM, NOTIFY THE ARCHITECT.
5. DO NOT SCALE DRAWINGS. USE FIGURED DIMENSIONS ONLY. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS. IN THE EVENT OF CONFLICT, NOTIFY ARCHITECT BEFORE PROCEEDING.
6. ALL ROOF JOISTS, TRUSSES, OUTLOOKERS, BEAMS AND GIRDER SHALL BE SECURED WITH APPROVED METAL TIES, CLIPS, AND ANCHORS TO TIE BEAMS.
7. ALL WOOD IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED.
8. USE GALVANIZED NAILS ONLY FOR EXTERIOR WOOD WORK.
9. ALL STUCCO SCRATCH COATS SHALL BE ALLOWED 24 HOURS DRYING PERIOD.
10. SUBSTITUTIONS OF SPECIFIED PRODUCTS AND / OR MATERIALS BY WRITTEN REQUEST TO AND SUBJECT TO APPROVAL BY OWNER/ARCHITECT.
11. CONCRETE: SEE STRUCTURAL DRAWINGS.
12. REINFORCING STEEL: SEE STRUCTURAL DRAWINGS.
13. STRUCTURAL STEEL: SEE STRUCTURAL DRAWINGS.
14. WOOD: SEE STRUCTURAL DRAWINGS.
15. MASONRY WALLS: SEE STRUCTURAL DRAWINGS.
16. COORDINATION: COORDINATE ALL DIMENSIONS, ELEVATIONS, OPENINGS, AND DETAILS WITH ARCHITECTURAL AND ELECTRICAL DRAWINGS. REPORT ANY CONFLICTS OR DISCREPANCIES TO OUR OFFICE. IN CASE OF CONFLICTS OR DISCREPANCIES, SUB-CONTRACTOR MUST ASSUME COSTLY SOLUTION DURING BIDDING UNLESS CLARIFICATION IS OBTAINED IN WRITING FROM THE ARCHITECT.
17. OWNER & CONTRACTOR NOTE: CHECKING OF SHOP DRAWINGS IS REQUIRED IF THIS OFFICE IS TO BE HELD RESPONSIBLE FOR THE STRUCTURAL ADEQUACY OF THE BUILDING.
18. SHORING, RE-SHORING & SAFETY PRACTICES: ALL SHORING AND RE-SHORING IS TO BE DESIGNED AND SPECIFIED BY FLORIDA LICENSED GENERAL CONTRACTOR. OUR OFFICE DOES NOT POSSESS, NOR PRESUMES TO POSSESS, ANY KNOWLEDGE OR EXPERTISE IN MATTERS RELATING TO JOB SAFETY, OSHA OR LABOR LAW REQUIREMENTS FOR A CONSTRUCTION PROJECT. SAFETY AND COMPLIANCE WITH OSHA AND LABOR LAWS IS THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND THOSE CONSULTANTS HE WIKES TO ADDRESS THEM.

Scope of Work

1. Addition (rear)
Ground floor: New master bedrooms (vaulted ceiling). See plans.
New master bath/closets and new kitchen. (See drawings)
Second floor: Loft over bath and kitchen open to bedroom.
Stucco and roof tile to match existing.
Total Construction (575 sf)
2. Remodeling
Removal of existing kitchen. New hallway to master bedroom with closets. Coordinate with drawings for bids.
3. Garage Addition
Extension to existing garage. Use same door. Stucco and roof tile to match existing. See construction documents.
Total Construction (91.33 sf)
4. Addition to Family Room.
See drawings. Total Construction (131 sf)
5. Addition Covered Terrace
See drawings. Total Construction (240 sf)

SAFEGUARDS

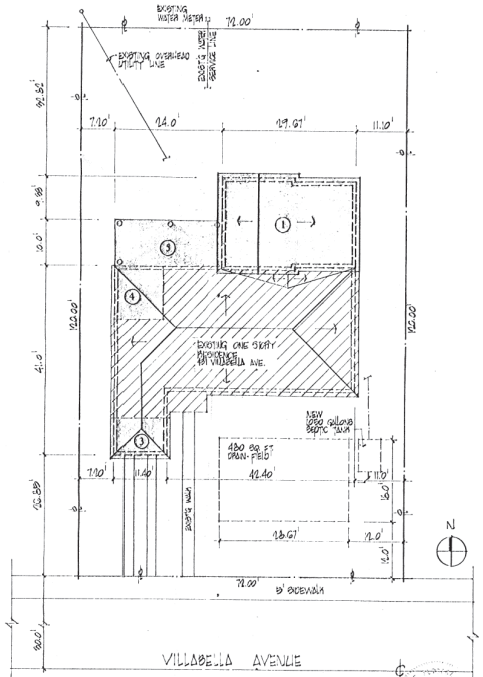
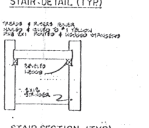
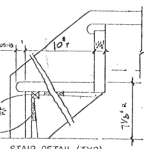
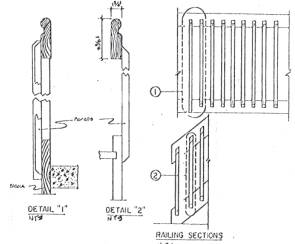
- (a) Railings, handrailings and other similar safeguards shall be designed to resist a load of 50 pounds per (linear) foot applied to any direction at the top of such barriers.
- (b) Intermediate rails, balusters, pickets and other fixtures shall be capable of resisting a uniform horizontal load over the entire area of not less than 25 pounds per square foot without restriction by deflection, but of not less strength than required to resist applicable wind loads as set forth herein.
- (c) The main supporting members of such metal barriers shall be designed and constructed to resist the forces set forth in Paragraphs (a) and (b) herein, whichever is critical, but the reaction of these forces need not be added.
- (d) Handrails shall be designed and constructed to resist a load of not less than 200 pounds applied in any direction at any point on the rail.

LEGAL DESCRIPTION:

431 VILLABELLA AVE. CORAL GABLES, FL. 33146
 LOT 15 1500 THE WEST 17.50' AND THE WEST
 31.00' OF LOT 6, BLOCK 23,
 MAP BOOK 23, PAGE 21
 DALLAS COUNTY, FLORIDA.

ZONING INFORMATION

1. Land Area	8,640 SQ FT
2. Lot Coverage Allowed	3,024 SQ FT
3. F.A.R. Allowed	3,614 SQ FT
F.A.R. Provided	4,280 SQ FT
4. Set Backs	
Front	10'-0"
Rear	31'-4"
Side (RIGHT)	11'-0"
Side (LEFT)	7'-0"
5. Building Height	10'-0"
6. Finish Floor Elevation at	11'-0"
7. Impervious Area Green Area	8,932 SQ FT
8. Total New Construction A/C	1,031 SQ FT



SITE PLAN 1/2"=10'

REVISIONS	BY

ADDITION & REMODELING
MILADY WHITE
 431 VILLABELLA AVE CORAL GABLES FLA.

JORGE L. BERNAL
 A. R. C. H. I. T. E. C. T.
Professional Seal Number: 12000 - State of Florida - Registered Professional Architect

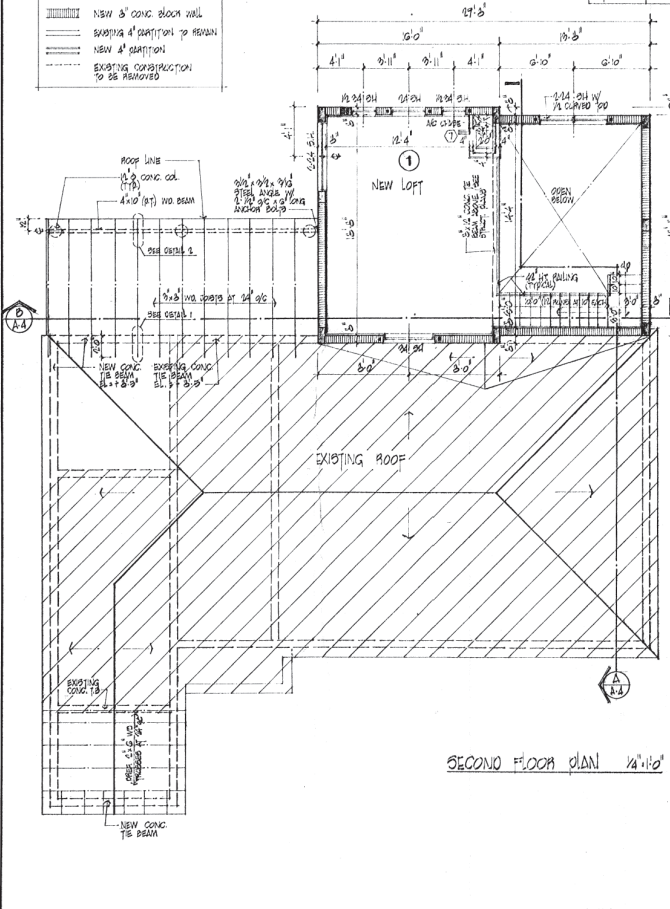
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 Title: 18 (3)mm
 Drawn: L.A.
 1/8"=10'
 Sheet: 1
 of 4 Sheets

FLOOR PLAN LEGEND

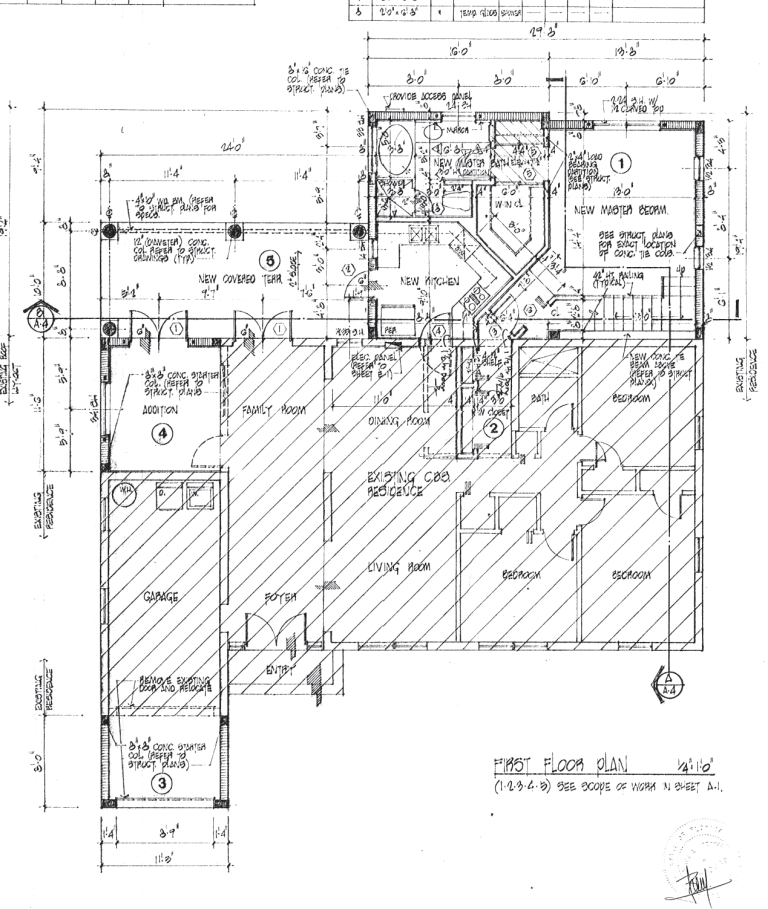
- EXISTING 3" CONC. BLOCK WALL TO REMAIN
- NEW 3" CONC. BLOCK WALL
- EXISTING 4" PARTITION TO REMAIN
- NEW 4" PARTITION
- EXISTING CONSTRUCTION TO BE REMOVED

AREA	FLOOR	FINISH		SCHEDULE		COL. INT.	WALL	REMARKS
		BASE	WALL	Ceiling	CL. INT.			
EXIST. RM								
WATER BATH								
WATER BATH								
KITCHEN								
LOFT								
COL. TRAIL								
EXPOSURE								

No.	SIZE	TYPICAL MATERIAL	TYPE		SCHEDULE		REMARKS
			NO.	NO.	NO.	NO.	
1	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
2	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
3	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
4	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
5	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
6	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
7	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
8	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
9	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
10	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
11	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
12	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
13	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
14	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
15	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
16	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
17	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
18	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
19	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
20	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
21	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
22	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
23	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
24	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
25	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
26	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
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29	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
30	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
31	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
32	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
33	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
34	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
35	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
36	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
37	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
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39	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
40	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
41	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
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43	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
44	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
45	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
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47	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
48	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
49	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
50	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
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52	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
53	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
54	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
55	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
56	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
57	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
58	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
59	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
60	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
61	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
62	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
63	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
64	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
65	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
66	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
67	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
68	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
69	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
70	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
71	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
72	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
73	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
74	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
75	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
76	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
77	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
78	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
79	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
80	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
81	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
82	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
83	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
84	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
85	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
86	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
87	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
88	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
89	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
90	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
91	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
92	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
93	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
94	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
95	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
96	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
97	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
98	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
99	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	
100	12'-0" x 12'-0"	WOOD	WOOD	WOOD	WOOD	WOOD	



SECOND FLOOR PLAN 1/4"=1'-0"

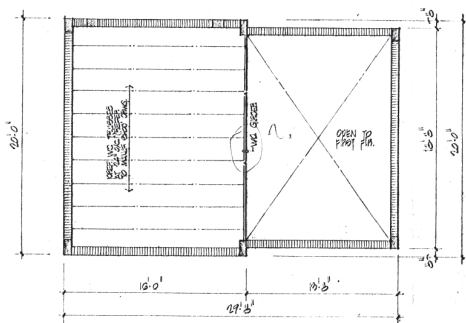


FIRST FLOOR PLAN 1/4"=1'-0"
(1-2-3-4-5) SEE SCOPE OF WORK IN SHEET A-1.

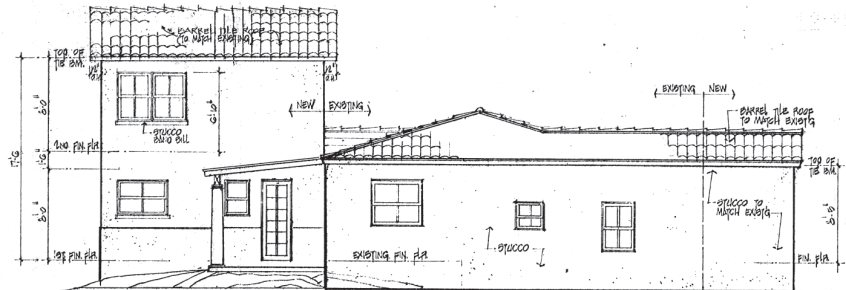
ADDITION & REMODELING
MILADY WHITE
 43 VILLABELLA AVE. CORAL GABLES, FLA.

FORCE L. BERNAL
 A. F. C. B. I. D. C. S.
 1200 South Bay Highway, Coral Gables, Florida 33134
 Phone: 305-441-1111

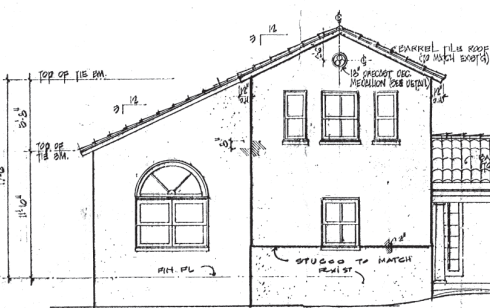
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 Job: M.W.
 Sheet: A-2
 Of: 4 Sheets



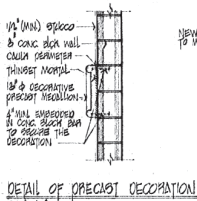
SECOND FLOOR FRAMING PLAN 1/4"=1'



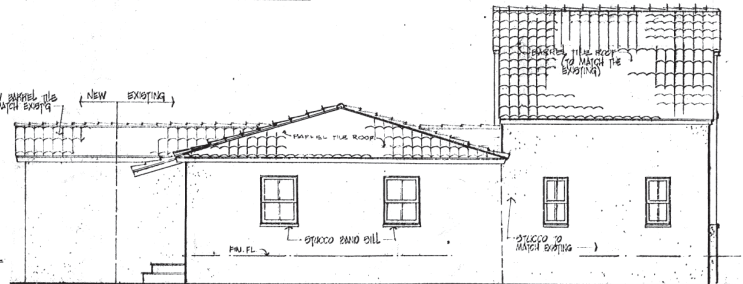
RIGHT ELEVATION 1/4"=1'



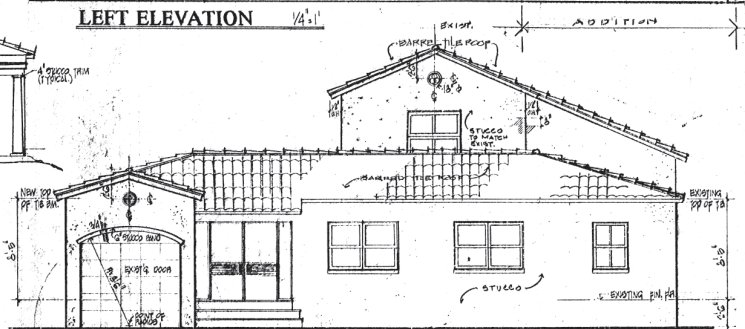
REAR ELEVATION 1/4"=1'



DETAIL OF ORECAST DECORATION 3/4"=1'



LEFT ELEVATION 1/4"=1'



FRONT ELEVATION 1/4"=1'

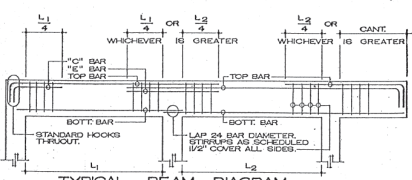
REVISIONS	BY

ADDITION & REMODELING
MILADY WHITE
 431 VILLABELLA AVE CORAL GABLES FLA.

JORGE L. BERNAL
 ARCHITECT
 1000 SW 15th Ave, Miami, FL 33135
 (305) 358-1111

Date: 11/24/14
 Size: 14' x 11' 0"
 Drawn:
 App: MWJ
 Scale: A-3
 Of 4 Sheets





COLUMN SCHEDULE				
MARK	SIZE	REINFORCING		REMARKS
		VERTICAL	TIES	
(1)	24 x 24	4#0	4#3 @ 12" o/c	
(2)	24 x 24	4#0	4#3 @ 12" o/c	
(3)	24 x 24	4#0	4#3 @ 12" o/c	

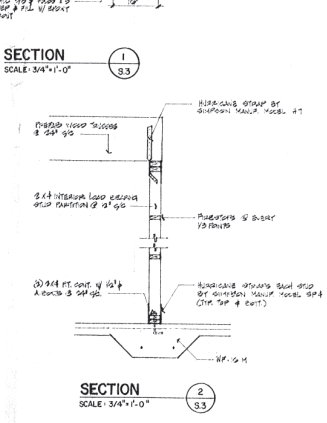
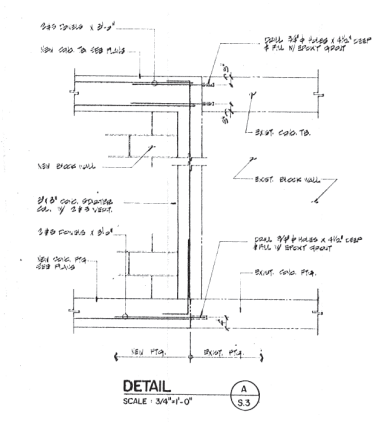
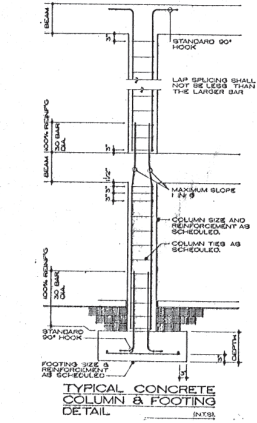
BEAM SCHEDULE						
MARK	TOP OF BEAM (ELEV.)	SIZE (IN.)	REINFORCEMENT			REMARKS
			B	T	C	
#B1	111'-0"	24" x 24"	1#9	1#9		3# 24" x 24", 12" @ 12"
#B2	111'-0"	24" x 24"	1#9	1#9		3# 24" x 24"
#B3	111'-0"	24" x 24"	1#9	1#9		3# 24" x 24"
#B4	111'-0"	24" x 24"	1#9	1#9		3# 24" x 24", 12" @ 12"
#B5	111'-0"	24" x 24"	1#9	1#9		3# 24" x 24", 12" @ 12"
#B6	111'-0"	24" x 24"	1#9	1#9		3# 24" x 24", 12" @ 12"
#B7	111'-0"	24" x 24"	1#9	1#9		3# 24" x 24", 12" @ 12"
#B8	111'-0"	24" x 24"	1#9	1#9		3# 24" x 24", 12" @ 12"
#B9	111'-0"	24" x 24"	1#9	1#9		3# 24" x 24", 12" @ 12"
#B10	111'-0"	24" x 24"	1#9	1#9		3# 24" x 24", 12" @ 12"

FOOTING SCHEDULE				
MARK	SIZE	REINFORCEMENT		REMARKS
		VERTICAL	TIES	
#F1	12" x 12"	4#0	4#0	24" x 24" x 12"
#F2	12" x 12"	4#0	4#0	24" x 24" x 12"
#F3	12" x 12"	4#0	4#0	24" x 24" x 12"
#F4	12" x 12"	4#0	4#0	24" x 24" x 12"
#F5	12" x 12"	4#0	4#0	24" x 24" x 12"
#F6	12" x 12"	4#0	4#0	24" x 24" x 12"
#F7	12" x 12"	4#0	4#0	24" x 24" x 12"
#F8	12" x 12"	4#0	4#0	24" x 24" x 12"
#F9	12" x 12"	4#0	4#0	24" x 24" x 12"
#F10	12" x 12"	4#0	4#0	24" x 24" x 12"

- GENERAL STRUCTURAL NOTES**
- ALLOWABLE SOIL BEARING CAPACITY.**
Based on a visual inspection of the site, the footings have been designed for a soil bearing capacity of 2,000 p.s.f.
 - CONCRETE.**
All concrete shall attain a minimum ultimate compressive strength of 3,000 psi in 28 days. Approximate ultimate compressive strength of concrete shall be 10% greater than the design strength. Concrete slump 3" minimum to 5" maximum.
 - CONCRETE COVER.**
To be as follows:

Position	Bottom	Top	Side
Slabs	1"	1"	1"
Beams	1 1/2"	1 1/2"	1 1/2"
Columns	1 1/2"	1 1/2"	1 1/2"
 - REINFORCING STEEL.**
To be new high strength billet steel deformed as per ASTM A-305, and conforming to ASTM A-635, Grade 60. Lap continuous bars 30-bar diameters.
 - MASONRY WALLS.**
Concrete block walls to be erected prior to placing concrete in adjacent beams. All concrete blocks to be grade 8-1, conforming to ASTM C-90 with a prism strength of 1,500 psi (minimum). Provide 9 gage ladder type joint reinforcing at every other course. Provide 1 # 3 x 5' long top and bottom course bars at all beam intersections, extend 50" minimum into each beam.
 - SLAB ON GROUND.**
Fill and backfill to be compacted to 95% of the maximum density at optimum moisture, as determined by the Standard Proctor Test. Compaction layers not to exceed 8". Slabs on fill to be placed on a checkerboard pattern, with pour limited to 600 sq. ft. or 25 feet in any direction.
 - STRUCTURAL STEEL.**
Shall conform to ASTM A-36, except for steel tubes which shall conform with ASTM A-500, Grade B (Fy = 46 ksi). Bolts to be A-325, Detailed, fabricated and erected in accordance with AISC specifications.
 - WOOD FRAMING.**
Must have a minimum fiber stress capacity (Fb) = 1,200 psi.
 - PREFABRICATED WOOD TRUSSES.**
To be manufactured in accordance with the South Florida Building Code, erected and braced according to the Truss Plate Institute's Specifications. Submit shop drawings signed and sealed by a Florida Professional Engineer for approval prior to fabrication.
Design Superimposed Loads:

Roof:	25 psf	15 psf
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 - REINFORCED MASONRY.**
Openings must be provided at all walls to be filled. Minimum height of grouting lifts shall be 8" - 10". All lateral reinforcing must have a minimum of 4# bar diameter with 18" spacing. All concrete used for grouting shall be precast type m, with a minimum strength of 3,000 psi. Mortar shall be Type M, with a minimum strength of 2,500 psi.
 - EXCAVATIONS.**
Adjacent to the existing structures. If there exists a deviation of the existing foundation from what is shown in these plans, contact architect and structural engineer. General contractor is solely responsible for the bracing and lateral support, and for maintaining the integrity of the existing structure during all phases of the construction.



REVISIONS BY

ADDITION & REMODELING
MILADY WHITE
 43 VILLABELLA AVE. CORAL GABLES, FLA.

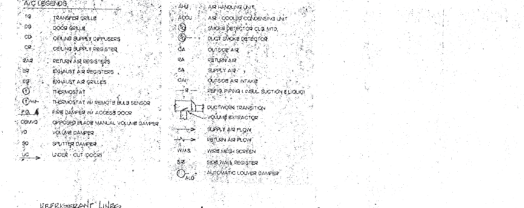
JORGE L. BERNAL
 A. C. C. H. I. C.
 1000 Biscayne Blvd., Suite 1200, Miami, Florida 33132
 Lic. No. 12000

DATE
 DRAWN
 JOB
 SHEET
S.3
 OF 3 SHEETS

AIR CONDITIONING SYSTEMS SCHEDULE																			
AIR HANDLING UNIT						AIR COOLED COND. UNIT SYSTEM													
UNIT NO.	CFM	ESP. IN. H ₂ O	HP	PLA. KW	ELECTRIC HEATING KW	MOODS	COMPRESSOR	TON	MAX. PLAC. SIZE	NO. OF UNITS	NO. OF LBS. COND.	NO. OF LBS. COND.	NO. OF LBS. COND.	NO. OF LBS. COND.	NO. OF LBS. COND.	NO. OF LBS. COND.	NO. OF LBS. COND.	NO. OF LBS. COND.	
1	800	0.5	1/4	1.7	4.3	1	TRIAL	11.0	11.0	12	12	12	12	12	12	12	12	12	12

NOTES
 1. OUTSIDE AIR DESIGN CONDITIONS 91 FDB - 79 FDB
 2. PROVIDE HEATING AND COOLING THERMOSTAT ON SUBBASE WITH FAN ON - OFF SWITCH
 3. SIZE AND RUN REFRIGERANT PIPING AS PER MANUFACTURER'S RECOMMENDATION
 4. PROVIDE VIBRATION ISOLATION WITH RUBBER IN CLEAR PADS
 5. INSULATE REFRIGERANT SUCTION LINE WITH 1" INSULATION
 6. INSTALL CHEMICAL DRYER AND SIGHT GLASS IN REFRIGERANT LIQUID LINE
 7. PROVIDE BUILT-IN DISCONNECT TO AIR HANDLING UNITS

GENERAL NOTES
 1. SEE ALL NOTES IN ALL OTHER RELATED DRAWINGS
 2. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 3. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 4. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 5. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 6. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 7. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 8. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 9. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 10. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 11. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 12. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 13. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 14. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 15. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 16. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 17. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 18. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 19. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING
 20. PROVIDE FINE MESH FILTERS IN ALL SUCTION PIPING



94126686

F/7739

Addition

OFFICE PLANS



95090385
431 Villa Bella

SECTION	BY	DATE
ARCHITECTURAL	[Signature]	12/11/14
MECHANICAL	[Signature]	12/11/14
ELECTRICAL	[Signature]	12/11/14
PLUMBING	[Signature]	12/11/14
STRUCTURAL	[Signature]	12/11/14
PAVE	[Signature]	12/11/14
LANDSCAPE	[Signature]	12/11/14
MARKING WORKS	[Signature]	12/11/14
ASPH. S.	[Signature]	12/11/14

RECEIVED 3 COPIES OF FLORIDA ENERGY EFFICIENCY FORMS.

NO WORK PROPOSED OUT RIGHT OF WAY

A266

RESIDENTIAL NON RESIDENTIAL

INDICATE THE TYPE OF FLOOD EDGE AND PROPOSED LOWEST FLOOR ELEVATION OR FLOOD PROOF ELEVATION IN RELATION TO MEAN SEA LEVEL IN ALL DISTRICTS.

OTHER 12/15 12/15

NEW CONSTRUCTION YES NO

SUBSTANTIAL IMPROVEMENTS YES NO

NOTE:
 ALL DAMAGED SIDEWALKS, CURBS AND/OR GUTTERS ARE TO BE REPLACED OR AS A RESULT OF CONSTRUCTION. REPAIRS TO BE COMPLETED PRIOR TO ISSUANCE OF CERTIFICATE OF OCCUPANCY. (ORD. NO. 2768)

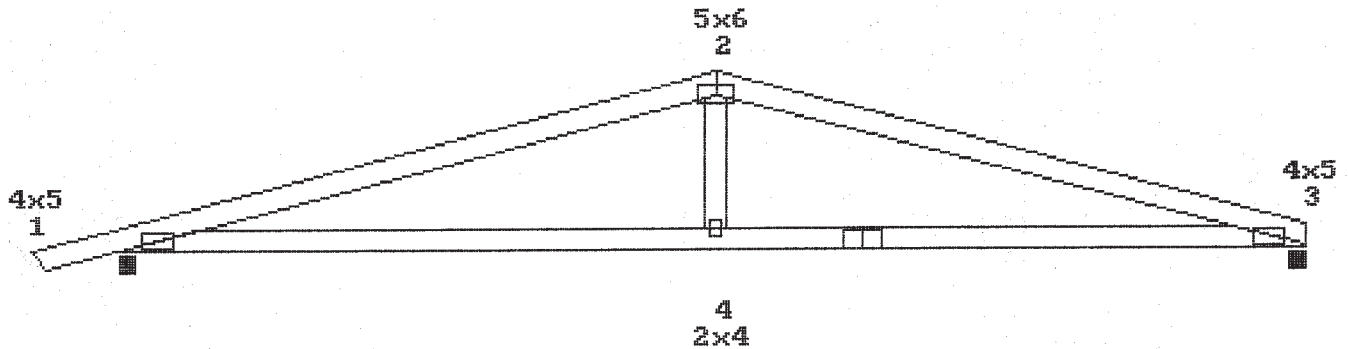
====<<<<A.C.E.S Version 7.2>>>>===== [001693] =====<<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 43.52 . Hand jig>>>>: 43.88 Hand assembly>>>>: 40.01
 Comp cut : 22.11 Comp assembly : 0.00
 Customer : MARK-IV Fri Mar. 15 15:31:02 1996
 Project #: 5K57 Truss ID : T1 Family #: 101
 Span : 16-0 Quantity : 9 Top Pitch : 5./12

FORCES - GRAVITY LOADS
 1-2=-1101 3-4= 1013 2-4= 158
 2-3=-1102 4-1= 1013

REACTIONS - SIZE
 1=-979 2.50
 3=-869 2.50



PROVIDE FOR 473 LBS UPLIFT AT JOINT 1 (0.48)
 PROVIDE FOR 342 LBS UPLIFT AT JOINT 3 (0.39)
 PROVIDE FOR 8 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:8-8-6 R. HL TO PK :8-7-10
 LEFT HEIGHT:0-6-3 SPAN:16-0 RISE:3-10-6 RIGHT HEIGHT:0-6-8

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	L 30 D 15	TOP	1-2=0.494	TOP CHORD:	2*6 No.2 19 SP
BOTT	0 10	BOTT	3-4=0.249	BOT CHORD:	2*6 No.2 19 SP
		LL.DEFL.	@4=0.02 < L/360	WEBS	:2*4 No.3 19 SP

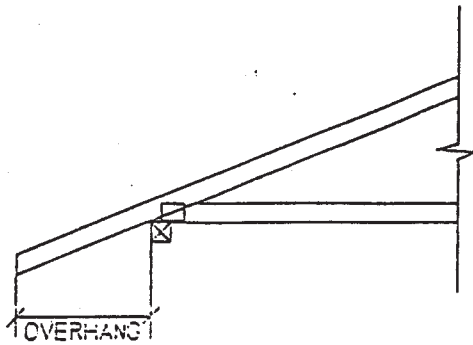
STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 17.5 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).
 PLATES ARE MITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS WITH NDS DESIGN SPECS, SSBC,TPI-91
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

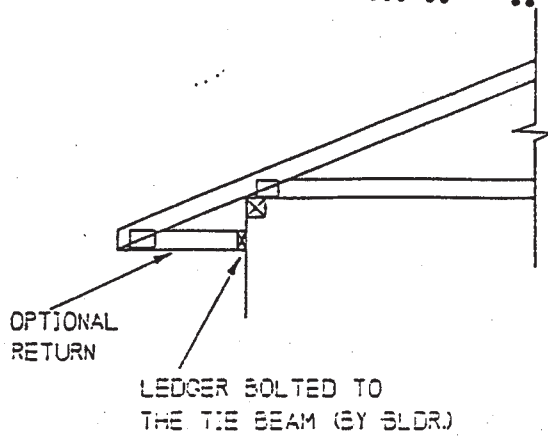
mm
 3-16-96

MAXIMUM OVERHANGS.

PREPARED BY QUAD OFFICE



OVERHANG WITHOUT RETURN



OVERHANG WITH RETURN

GRADE OF LUMBER	10 PSF SOFFIT		2 PSF SOFFIT	
	2X4	2X6	2X4	2X6
No.2ND 19 SP	3-1-12	4-6-12	3-6-4	5-1-4
No.2 19 SP	3-3-13	4-9-2	3-8-8	5-3-14
No.2D 19 SP	3-6-6	5-1-8	3-11-6	5-8-12
No.1ND 19 SP	3-6-6	5-2-9	3-11-6	5-9-15
No.1 19 SP	3-8-3	5-5-10	4-1-7	6-1-6
No.1D 19 SP	3-9-15	5-7-9	4-3-6	6-3-9
NDSS 19 SP	4-1-7	6-5-11	4-8-10	7-3-9
SS 19 SP	4-2-7	6-7-3	4-9-11	7-6-10
DSS 19 SP	4-3-5	6-8-10	4-10-12	7-8-4

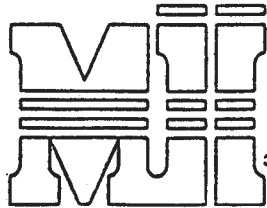
MINIMUM GRADE OF LUMBER

T.C. 2X4 No.2ND 19 SP
 B.C. 2X4 No.2ND 19 SP
 Webs 2X4 No.3 19 SP

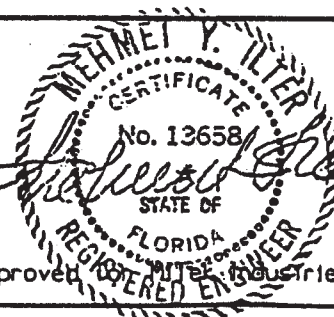
TPI-92 Cr11.
 SSBC-91

LOADING (PSF)
 L D
 TOP 20 10
 BOTTOM 0 10
 SPACING: 24 Inch O.C.


STR. INCR. : 15 %
 DRAWN BY : U.A.
 CHECKED BY : JAI
 REP. STRESS: YES





MiTek Industries Inc.



Approved: MiTek Industries Inc.

 This safety alert symbol is used to attract your attention! **PERSONAL SAFETY IS INVOLVED!** When you see this symbol - **BECOME ALERT - HEED ITS MESSAGE.**

 **CAUTION:** A CAUTION identifies safe operating practices or indicates unsafe conditions that could result in personal injury or damage to structures.

 **DANGER:** A DANGER designates a condition where failure to follow instructions or heed warning will most likely result in serious personal injury or death or damage to structures.

 **WARNING:** A WARNING describes a condition where failure to follow instructions could result in severe personal injury or damage to structures.


HIB-91 Summary Sheet


COMMENTARY and RECOMMENDATIONS for HANDLING, INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES[®]


TRUSS PLATE INSTITUTE
583 D'Onofrio Dr., Suite 200
Madison, Wisconsin 53719
(608) 833-5900

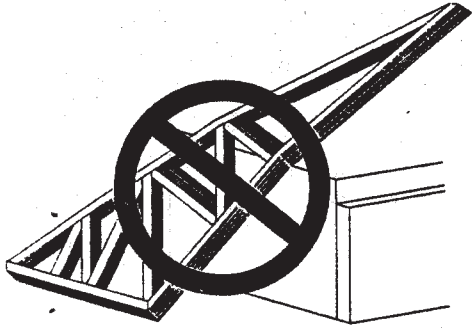
It is the responsibility of the installer (builder, building contractor, licensed contractor, erector or erection contractor) to properly receive, unload, store, handle, install and brace metal plate connected wood trusses to protect life and property. The installer must exercise the same high degree of safety awareness as with any other structural material. TPI does not intend these recommendations to be interpreted as superior to the project Architect's or Engineer's design specification for handling, installing and bracing wood trusses for a particular roof or floor. These recommendations are based upon the collective experience of leading technical


personnel in the wood truss industry, but must, due to the nature of responsibilities involved, be presented as a guide for the use of a qualified building designer or installer. Thus, the Truss Plate Institute, Inc. expressly disclaims any responsibility for damages arising from the use, application or reliance on the recommendations and information contained herein by building designers, installers, and others. Copyright © by Truss Plate Institute, Inc. All rights reserved. This document or any part thereof must not be reproduced in any form without written permission of the publisher. Printed in the United States of America.

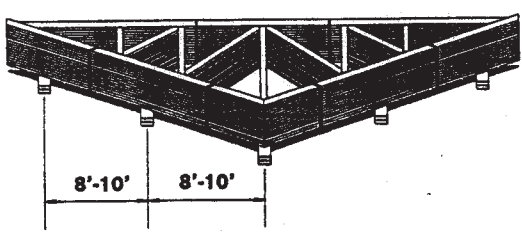
 **CAUTION:** The builder, building contractor, licensed contractor, erector or erection contractor is advised to obtain and read the entire booklet *Commentary and Recommendations for Handling, Installing & Bracing Metal Plate Connected Wood Trusses, HIB-91[®]* from the Truss Plate Institute.

 **CAUTION:** All temporary bracing should be no less than 2x4 grade marked lumber. All connections should be made with minimum of 2-16d nails. All trusses assumed 2' on-center or less. All multi-ply trusses should be connected together in accordance with design drawings prior to installation.

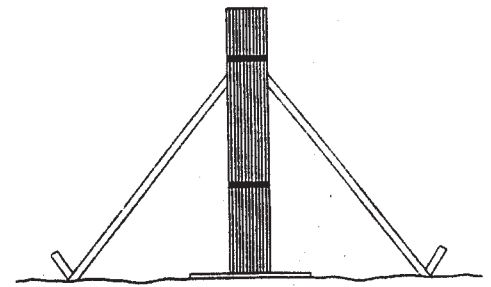
TRUSS STORAGE



 **CAUTION:** Trusses should not be unloaded on rough terrain or uneven surfaces which could cause damage to the truss.



Trusses stored horizontally should be supported on blocking to prevent excessive lateral bending and lessen moisture gain.




Trusses stored vertically should be braced to prevent toppling or tipping.

 **WARNING:** Do not break banding until installation begins or lift bundled trusses by the bands.

 **DANGER:** Do not store bundles upright unless properly braced.

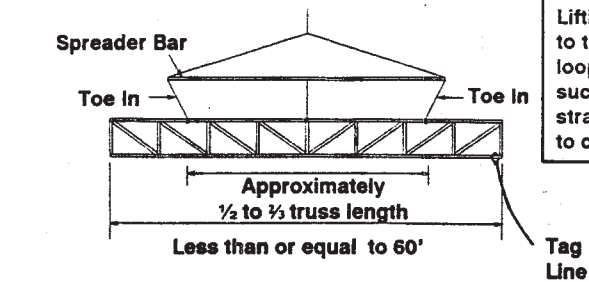
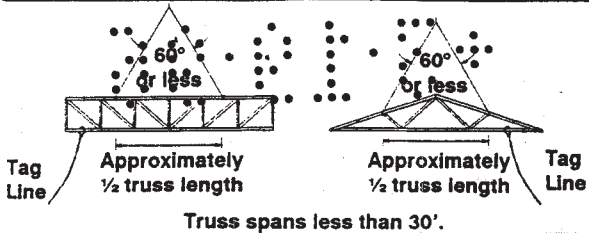
 **WARNING:** Do not use damaged trusses.

 **DANGER:** Walking on trusses which are lying flat is extremely dangerous and should be strictly prohibited.

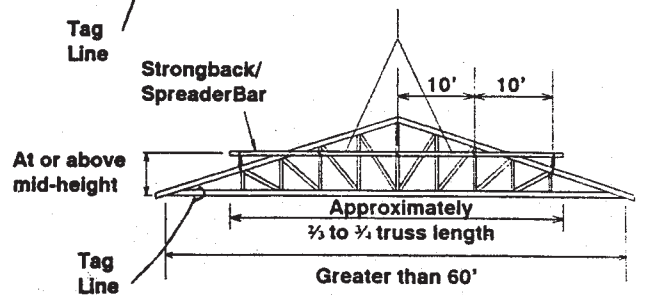
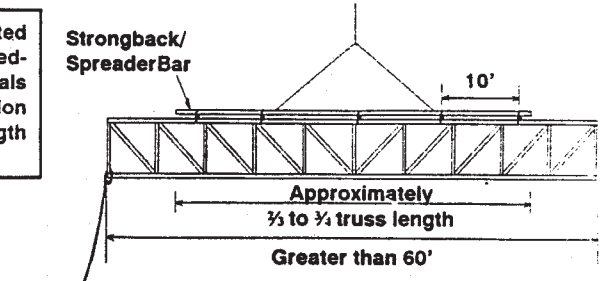
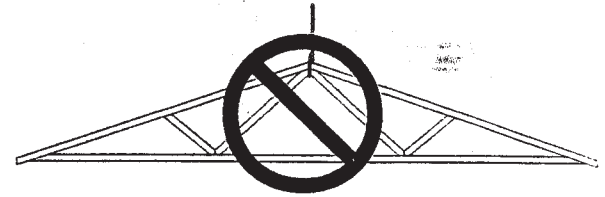
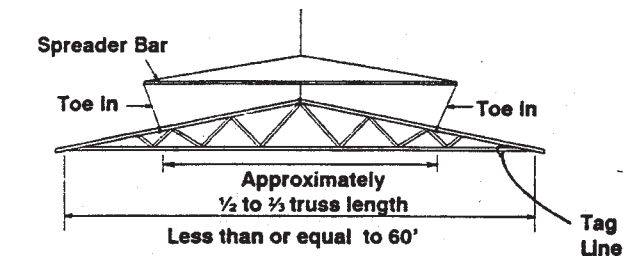
WARNING: Do not attach cables, chains, or hooks to the web members.

WARNING: Do not lift single trusses with spans greater than 30' by the peak.

MECHANICAL INSTALLATION



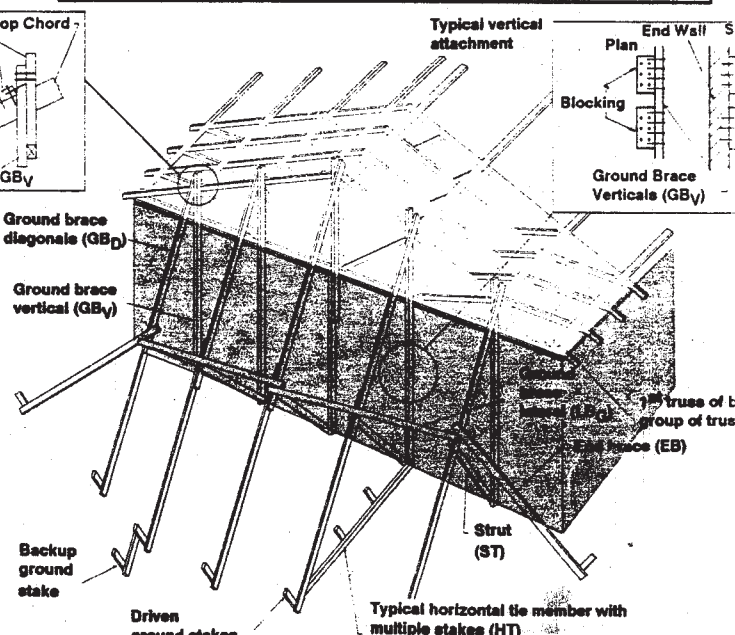
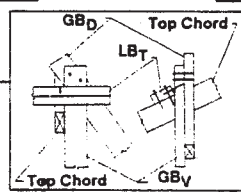
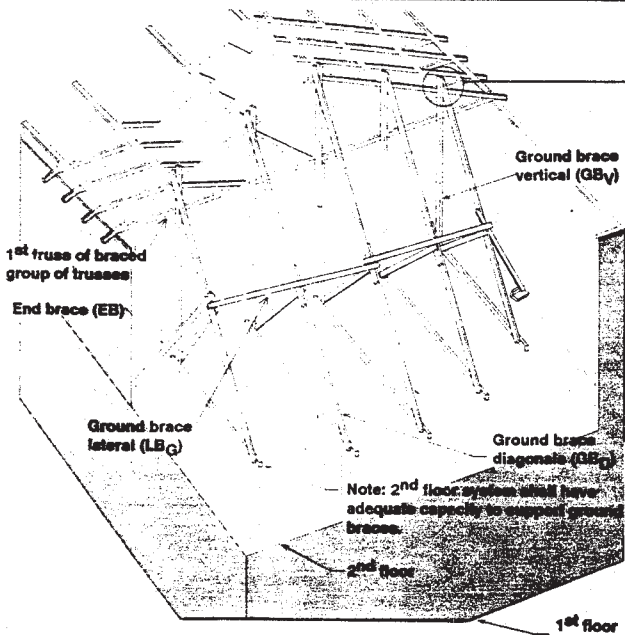
Lifting devices should be connected to the truss top chord with a closed-loop attachment utilizing materials such as slings, chains, cables, nylon strapping, etc. of sufficient strength to carry the weight of the truss.



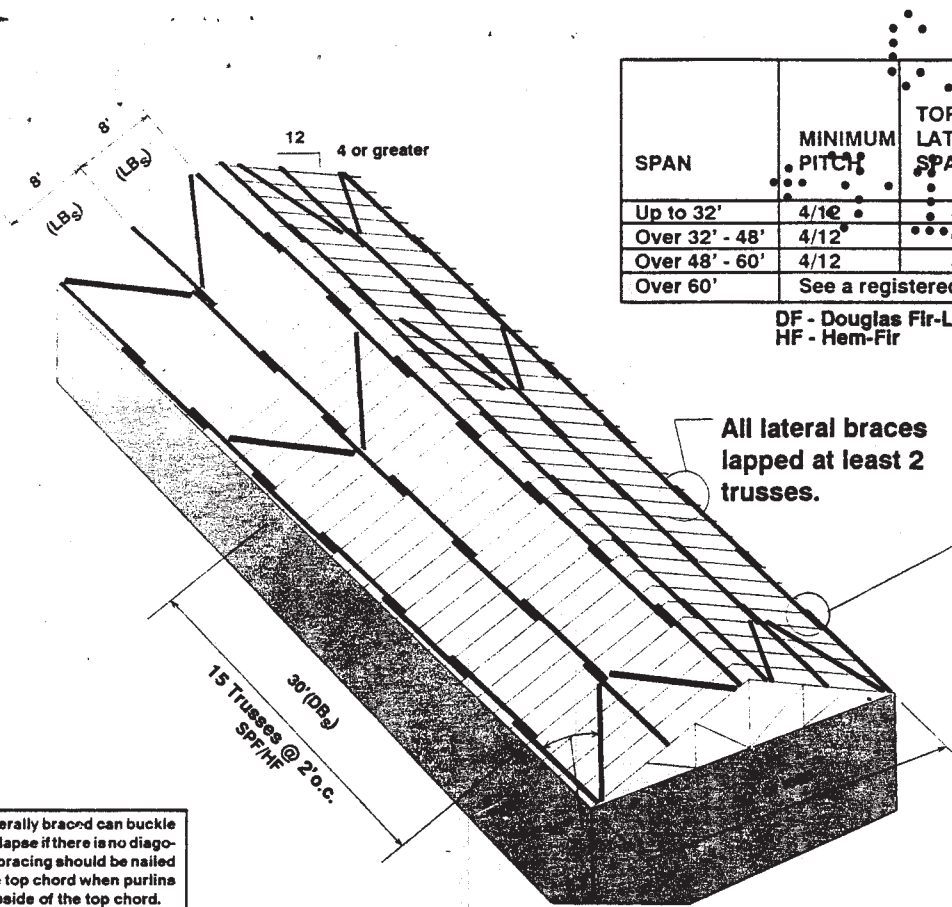
CAUTION: Temporary bracing shown in this summary sheet is adequate for the installation of trusses with similar configurations. Consult a registered professional engineer if a different bracing arrangement is desired. The engineer may design bracing in accordance with TPI's *Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses, DSB-89*, and in some cases determine that a wider spacing is possible.

GROUND BRACING: BUILDING INTERIOR

GROUND BRACING: BUILDING EXTERIOR



CAUTION: Ground bracing required for all installations.

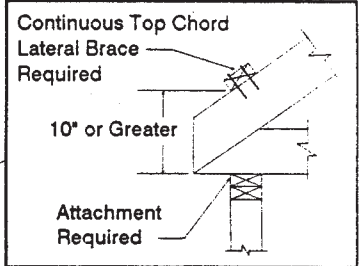


SPAN	MINIMUM PITCH	TOP CHORD LATERAL BRACE SPACING (LB _s)	TOP CHORD DIAGONAL BRACE SPACING (DB _s)	
			[# trusses]	
			SP/DF	SPF/HF
Up to 32'	4/12	8'	20	15
Over 32' - 48'	4/12	6'	10	7
Over 48' - 60'	4/12	5'	6	4
Over 60'	See a registered professional engineer			

DF - Douglas Fir-Larch
HF - Hem-Fir

SP - Southern Pine
SPF - Spruce-Pine-Fir

All lateral braces lapped at least 2 trusses.



Top chords that are laterally braced can buckle together and cause collapse if there is no diagonal bracing. Diagonal bracing should be nailed to the underside of the top chord when purlins are attached to the topside of the top chord.

PITCHED TRUSS

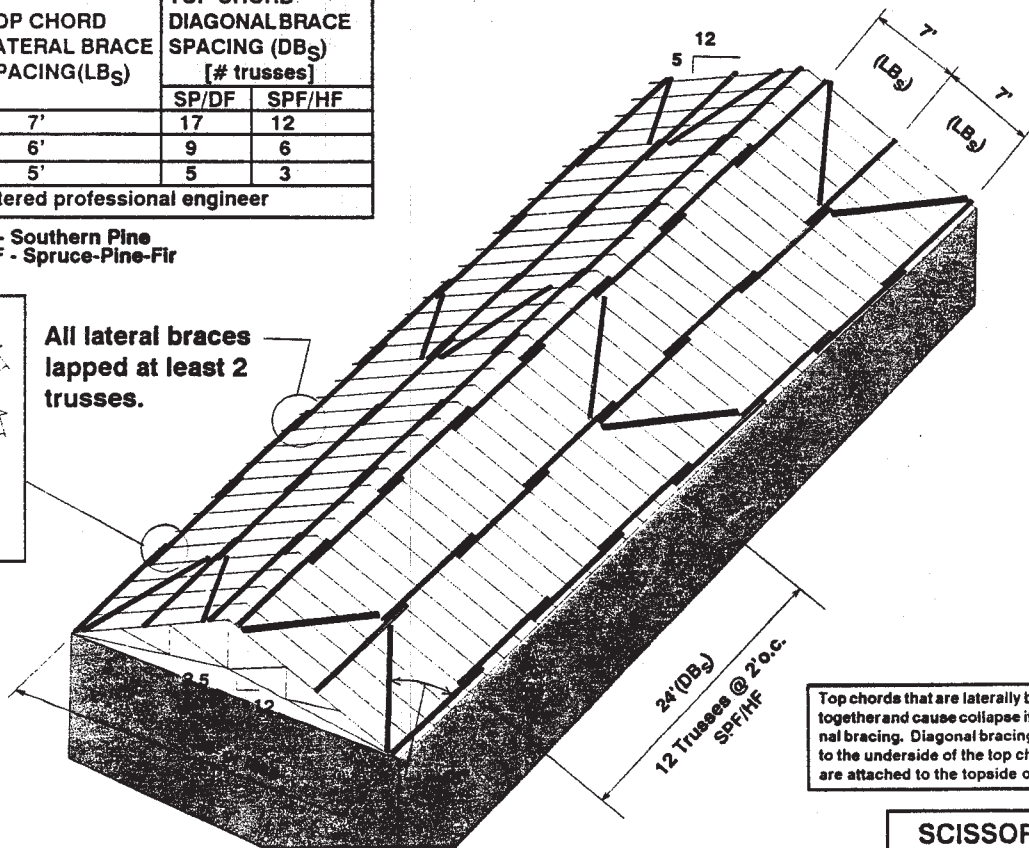
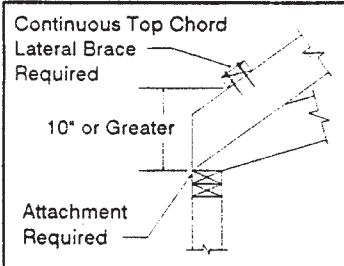
WARNING: Failure to follow these recommendations could result in severe personal injury or damage to trusses or buildings.

SPAN	MINIMUM PITCH DIFFERENCE	TOP CHORD LATERAL BRACE SPACING (LB _s)	TOP CHORD DIAGONAL BRACE SPACING (DB _s)	
			[# trusses]	
			SP/DF	SPF/HF
Up to 28'	2.5	7'	17	12
Over 28' - 42'	3.0	6'	9	6
Over 42' - 60'	3.0	5'	5	3
Over 60'	See a registered professional engineer			

DF - Douglas Fir-Larch
HF - Hem-Fir

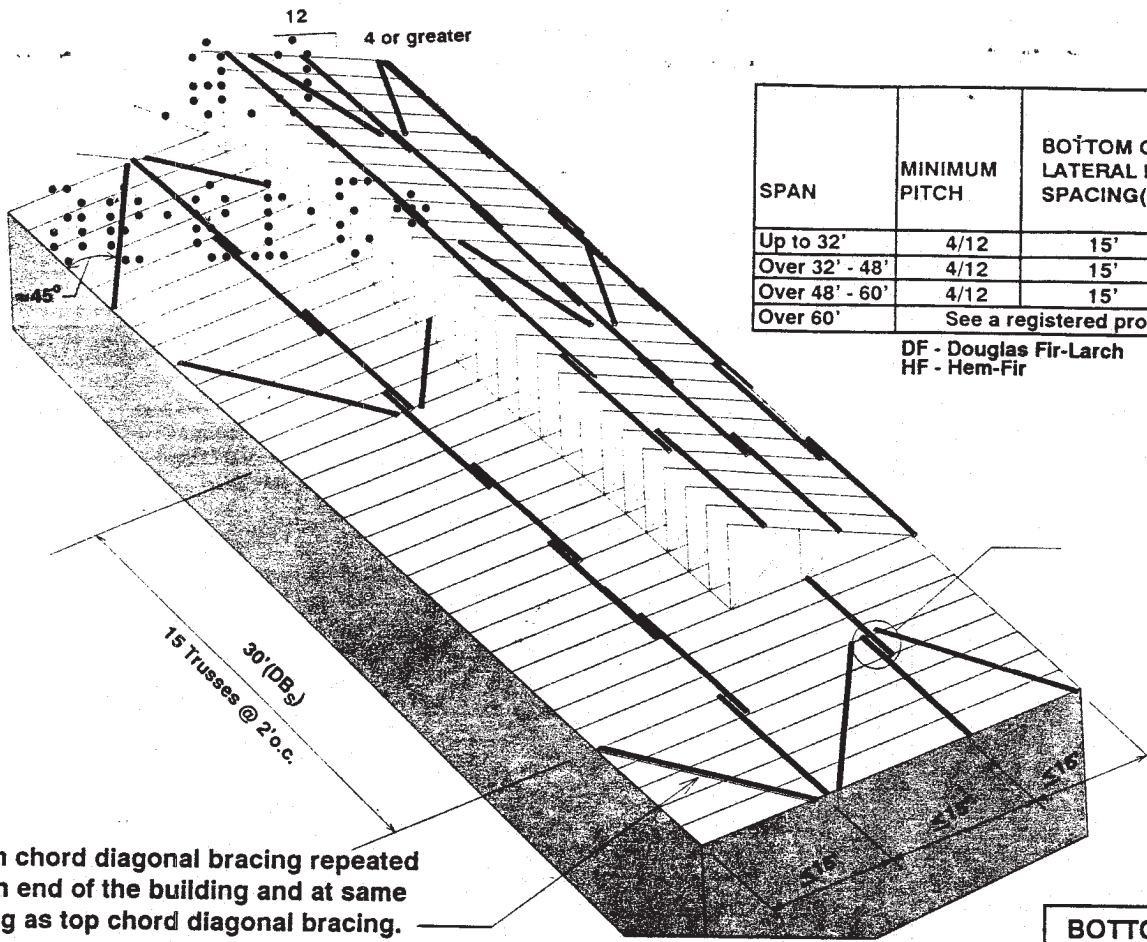
SP - Southern Pine
SPF - Spruce-Pine-Fir

All lateral braces lapped at least 2 trusses.



Top chords that are laterally braced can buckle together and cause collapse if there is no diagonal bracing. Diagonal bracing should be nailed to the underside of the top chord when purlins are attached to the topside of the top chord.

SCISSORS TRUSS



SPAN	MINIMUM PITCH	BOTTOM CHORD LATERAL BRACE SPACING (LB _S)	BOTTOM CHORD DIAGONAL BRACE SPACING (DB _S) [# trusses]	
			SP/DF	SPF/HF
Up to 32'	4/12	15'	20	15
Over 32' - 48'	4/12	15'	10	7
Over 48' - 60'	4/12	15'	6	4
Over 60'	See a registered professional engineer			

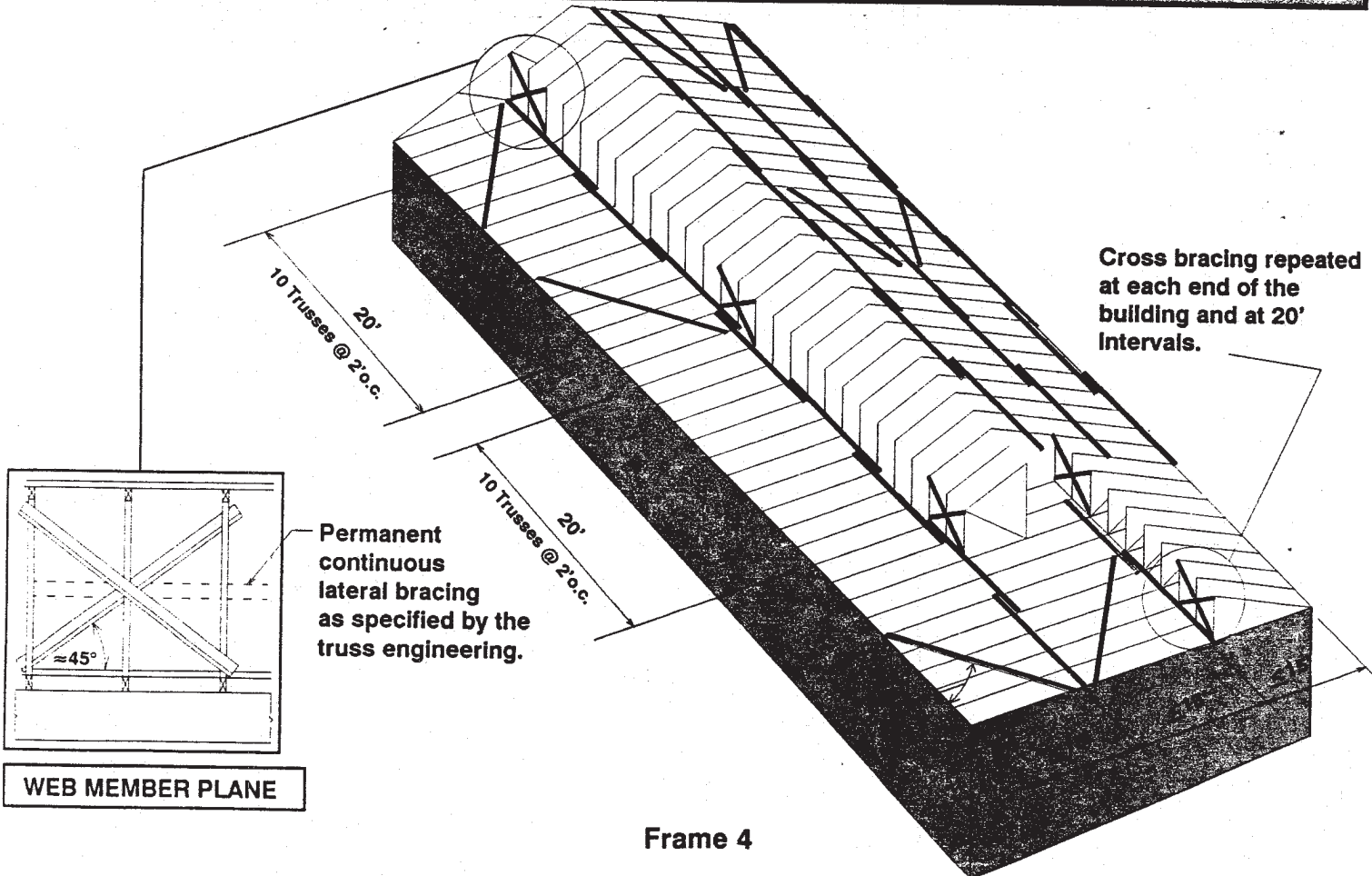
DF - Douglas Fir-Larch
HF - Hem-Fir

SP - Southern Pine
SPF - Spruce-Pine-Fir

Bottom chord diagonal bracing repeated at each end of the building and at same spacing as top chord diagonal bracing.

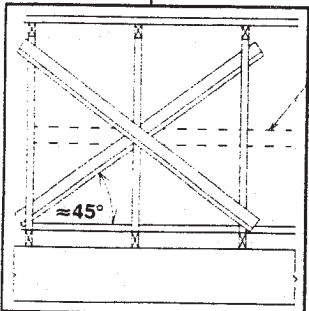
All lateral braces lapped at least 2 trusses.

BOTTOM CHORD PLANE



Cross bracing repeated at each end of the building and at 20' intervals.

Permanent continuous lateral bracing as specified by the truss engineering.



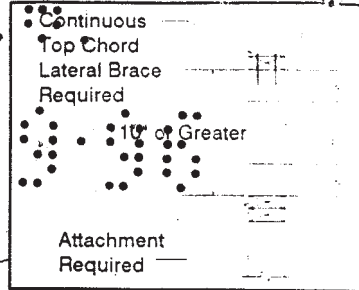
WEB MEMBER PLANE

Frame 4

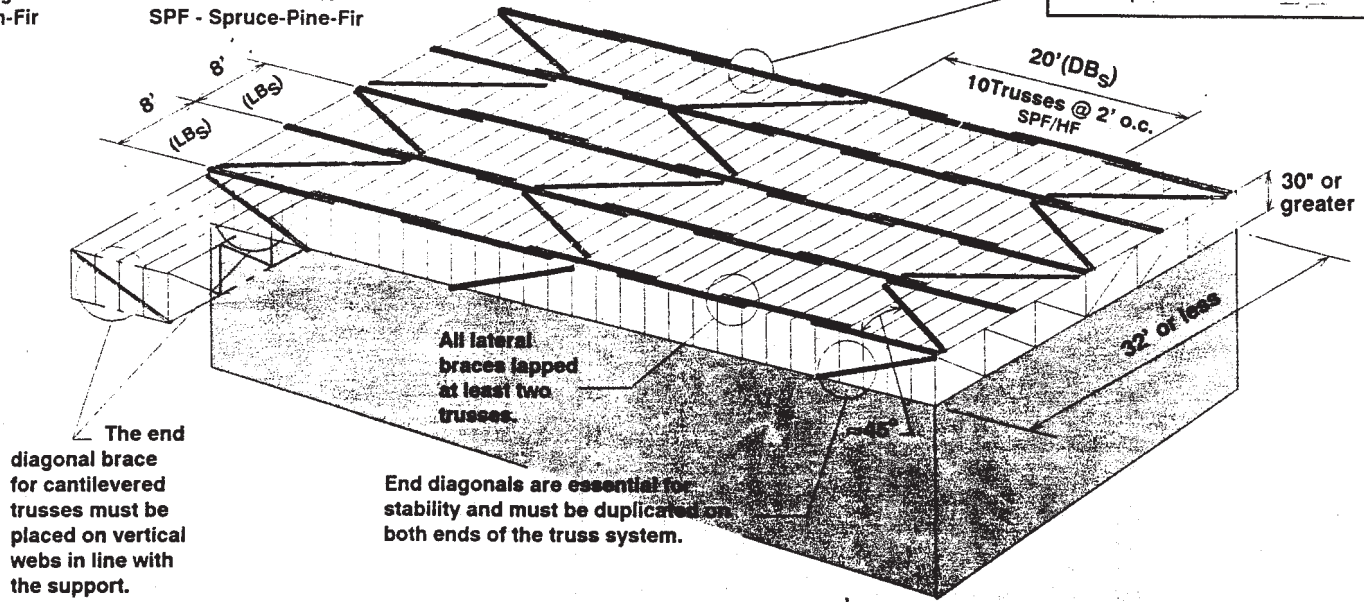
SPAN	MINIMUM DEPTH	TOP CHORD LATERAL BRACE SPACING (LB _s)	TOP CHORD DIAGONAL BRACE SPACING (DB _s)	
			[# trusses]	
			SP/DF	SPF/HF
Up to 32'	30"	8'	16	10
Over 32' - 48'	42"	6'	6	4
Over 48' - 60'	48"	5'	4	2
Over 60'	See a registered professional engineer			

2x4/2x6 PARALLEL CHORD TRUSS

Top chords that are laterally braced can buckle together and cause collapse if there is no diagonal bracing. Diagonal bracing should be nailed to the underside of the top chord when purlins are attached to the topside of the top chord.

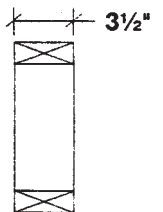
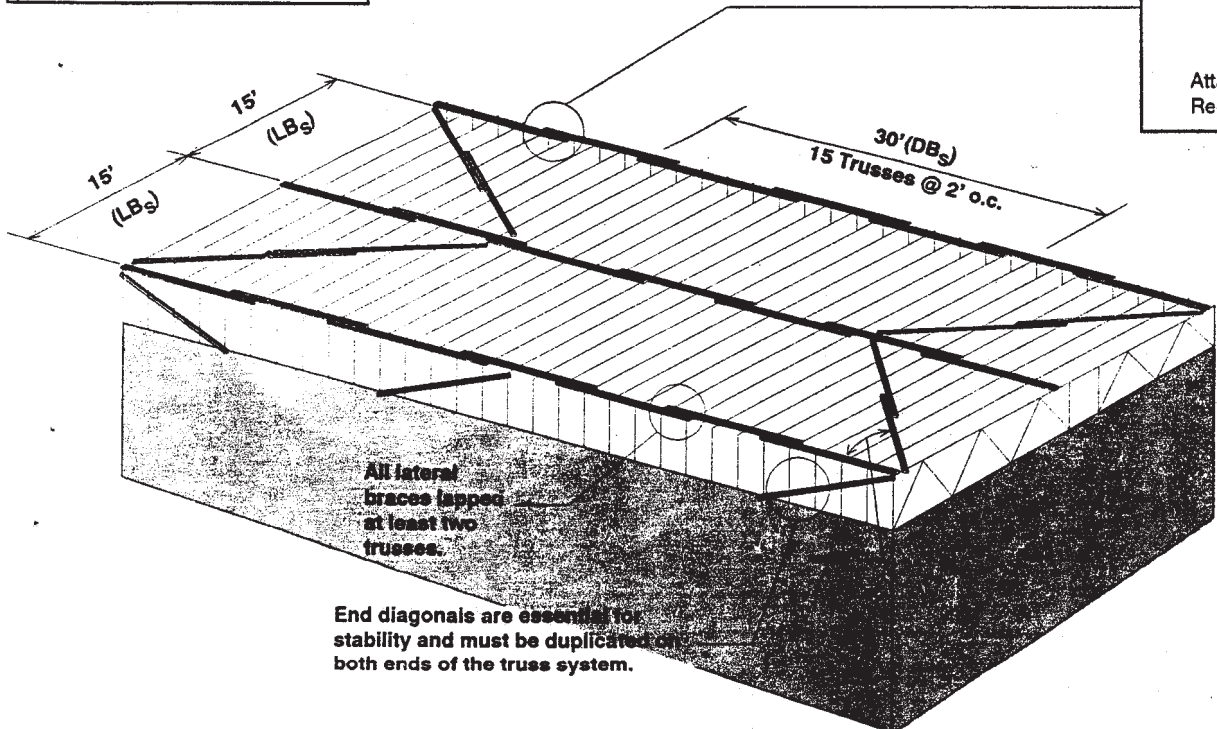
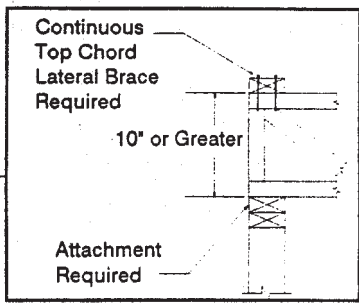


DF - Douglas Fir-Larch SP - Southern Pine
HF - Hem-Fir SPF - Spruce-Pine-Fir



4x2 PARALLEL CHORD TRUSS: TOP CHORD

Top chords that are laterally braced can buckle together and cause collapse if there is no diagonal bracing. Diagonal bracing should be nailed to the underside of the top chord when purlins are attached to the topside of the top chord.

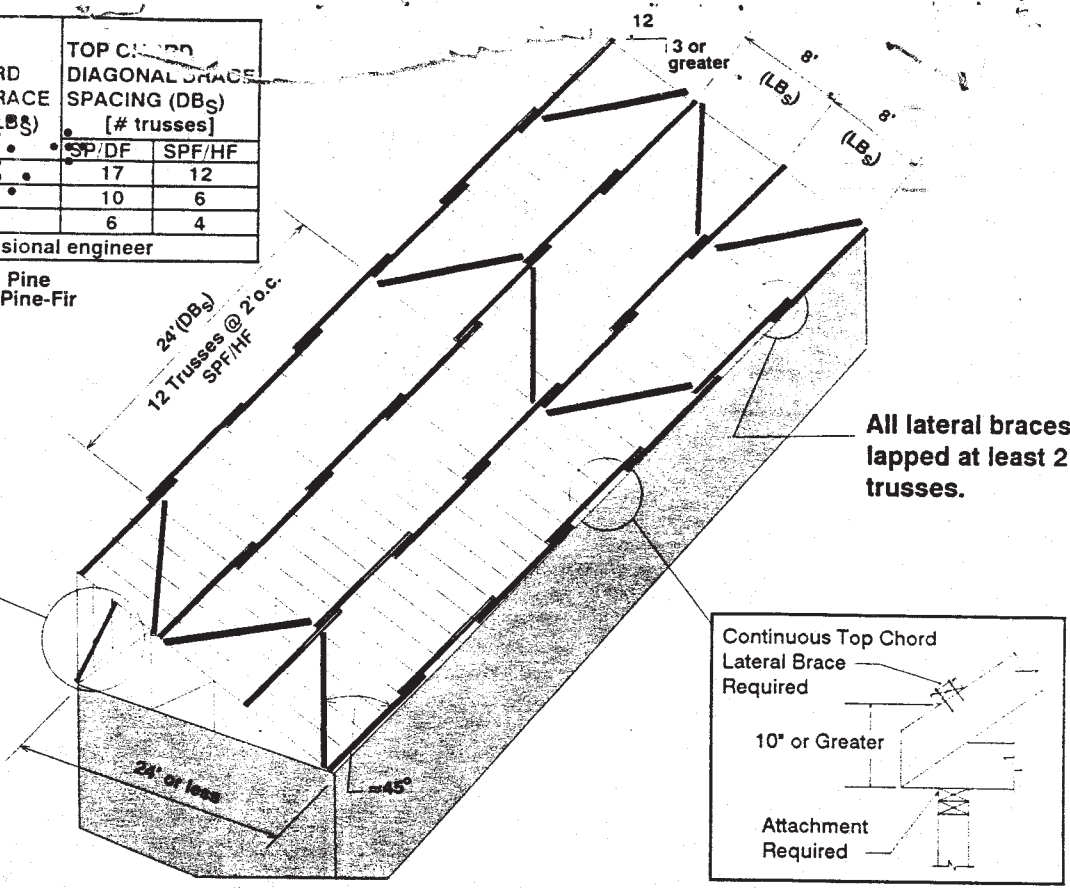


Trusses must have lumber oriented in the horizontal direction to use this brace spacing.

SPAN	MINIMUM PITCH	TOP CHORD LATERAL BRACE SPACING (LBS)	TOP CHORD DIAGONAL BRACE SPACING (DBs) [# trusses]	
			SP/DF	SPF/HF
Up to 24'	3/12	8'	17	12
Over 24' - 42'	3 1/2	7'	10	6
Over 42' - 54'	3/12	6'	6	4
Over 54'	See a registered professional engineer			

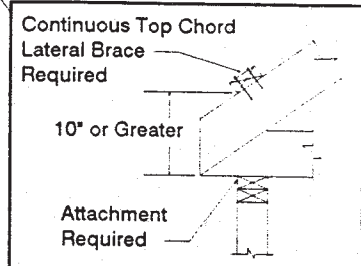
DF - Douglas Fir-Larch
HF - Hem-Fir

SP - Southern Pine
SPF - Spruce-Pine-Fir



Diagonal brace also required on end verticals.

Top chords that are laterally braced can buckle together and cause collapse if there is no diagonal bracing. Diagonal bracing should be nailed to the underside of the top chord when purlins are attached to the topside of the top chord.



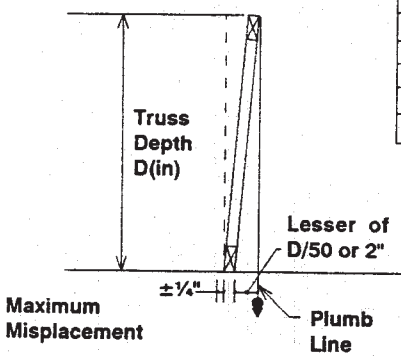
MONO TRUSS

WARNING: Failure to follow these recommendations could result in severe personal injury or damage to property or equipment.

INSTALLATION TOLERANCES

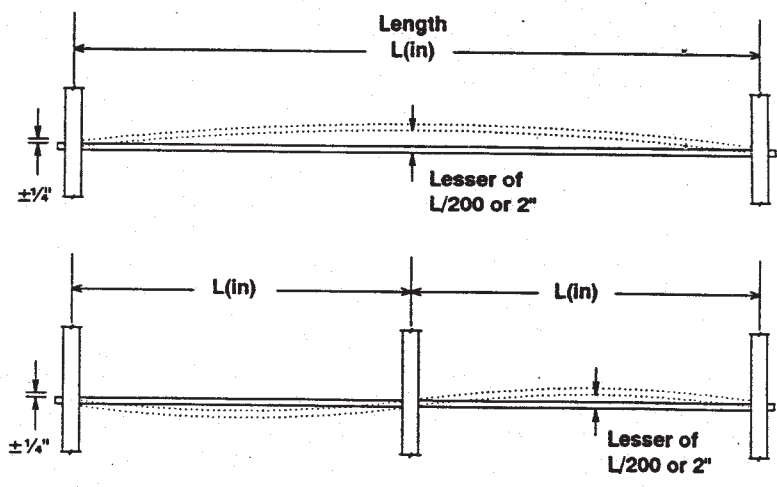
PLUMB

D(in)	D/50	D(ft)
12"	1/4"	1'
24"	1/2"	2'
36"	3/4"	3'
48"	1"	4'
60"	1-1/4"	5'
72"	1-1/2"	6'
84"	1-3/4"	7'
96"	2"	8'
108"	2"	9'



OUT-OF-PLUMB INSTALLATION TOLERANCES.

BOW



L(in)	L/200	L(ft)
50"	1/4"	4.2'
100"	1/2"	8.3'
150"	3/4"	12.5'

L(in)	L/200	L(ft)
200"	1"	16.7'
250"	1-1/4"	20.8'
300"	1-1/2"	25.0'

OUT-OF-PLANE INSTALLATION TOLERANCES.

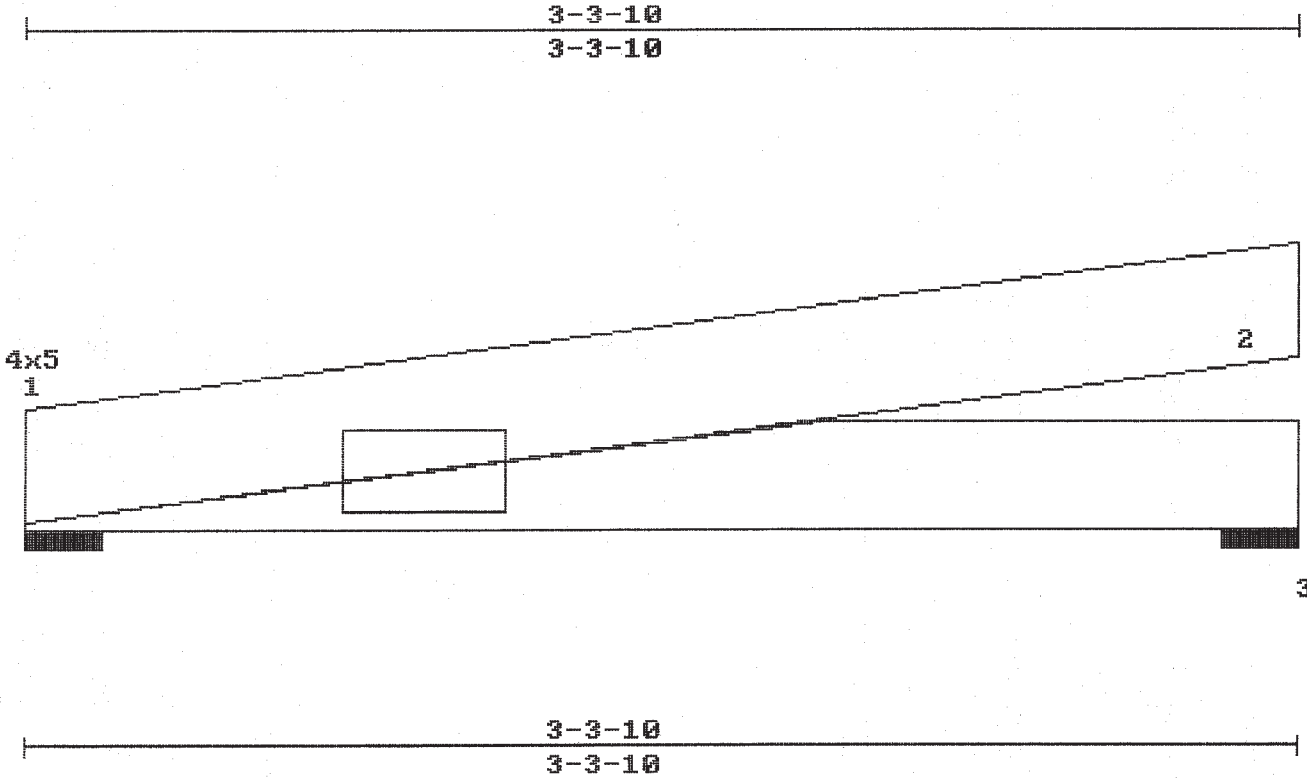
WARNING: Do not install trusses out of plumb.

WARNING: Do not install trusses out of plumb.

====<<<<A.C.E.S Version 7.2>>>>===== [001678] =====<<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 1.45 . Hand jig>>>>: 2.13 Hand assembly>>>>: 0.91
 Comp cut : 0.50 Comp assembly : 0.00
 Customer : MARK-IV Fri Mar 15 15:27:24 1996
 Project #: 5K57 Truss ID : CJ24 Family #: 204
 Span : 3-3-10 Quantity : 1 Top Pitch : 2.475/12

FORCES - LOAD CASE #1 REACTIONS - SIZE
 1-2= 0 3-1= 0 1=-70 2.50
 2-3=-57 3=-70 2.50

PROVIDE FOR 104 LBS UPLIFT AT JOINT 1 (1.48)
 PROVIDE FOR 116 LBS UPLIFT AT JOINT 3 (1.66)
 PROVIDE FOR 56 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:3-4-7
 LEFT HEIGHT:0-6 SPAN:3-3-10 RISE:1-2-3 RIGHT HEIGHT:1-2-3

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD:	No. 2 19 SP
30	15	1-2=0.089	3-1=0.010	BOT CHORD:	No. 2 19 SP
0	10	LL.DEFL. < L/360		WEBS	:2*4 No. 3 19 SP

REPETITIVE STRESSES NOT USED SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 2= 37 3- 1= 8

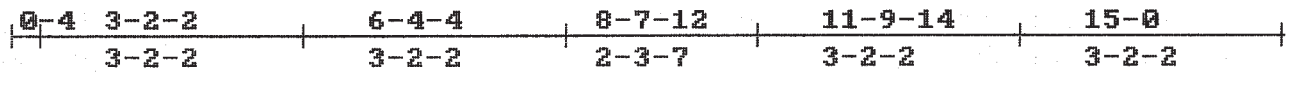
TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 10 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).
 PLATES ARE MITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS WITH NDS DESIGN SPECS, SSBC,TPI-91
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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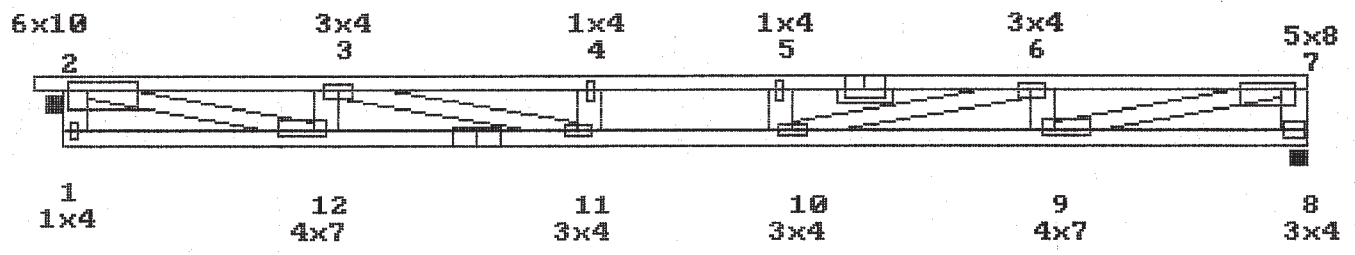
====<<<<A.C.E.S Version 7.2>>>>===== [001679] =====<<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 22.17 Hand jig>>>>: 24.00 Hand assembly>>>>: 19.00
 Comp cut : 10.50 Comp assembly : 0.00
 Customer : MARK-IV Fri Mar 15 15:27:34 1996
 Project #: 5K57 Truss ID : FF1 Family #: 303
 Span : 15-0 Quantity : 5 Top Pitch : /12..

REACTIONS - SIZE
 2=-893 2.50
 8=-819 2.50

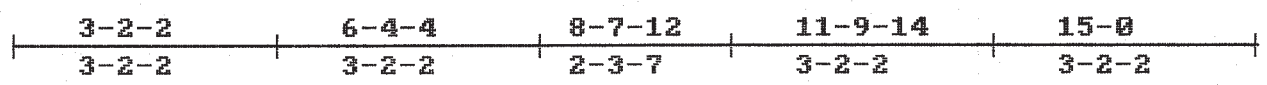
PLATE OFFSETS (X=LEFT,Y=TOP): [j2=2.5,2], [j7=6,1.5], [j9=2,2.5], [j12=5,2.5],



5x6 SPL.
 W/B



4x7 SPL.



LEFT HEIGHT:1-4 SPAN:15-0 RISE:1-4 RIGHT HEIGHT:1-4

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	3-4=0.335	TOP CHORD:	2*4 No.2 19 SP
TOP	40 10	BOTT	10-11=0.767	BOT CHORD:	2*4 No.2 19 SP
BOTT	0 5	LL.DEFL.@10	=0.18 < L/360	WEBS	:2*4 No.3 19 SP

STR.INC.: LUMB = 1.00 PLATE = 1.00 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

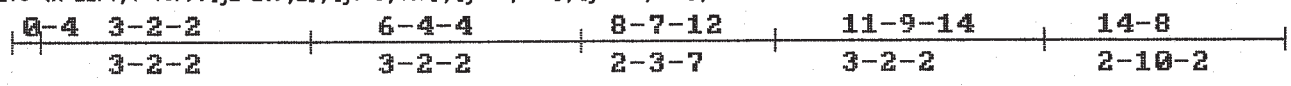
BOTH END VERTICALS ARE NOT EXPOSED.
 PLATES ARE MITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS WITH NDS DESIGN SPECS, SSBC,NDS-91
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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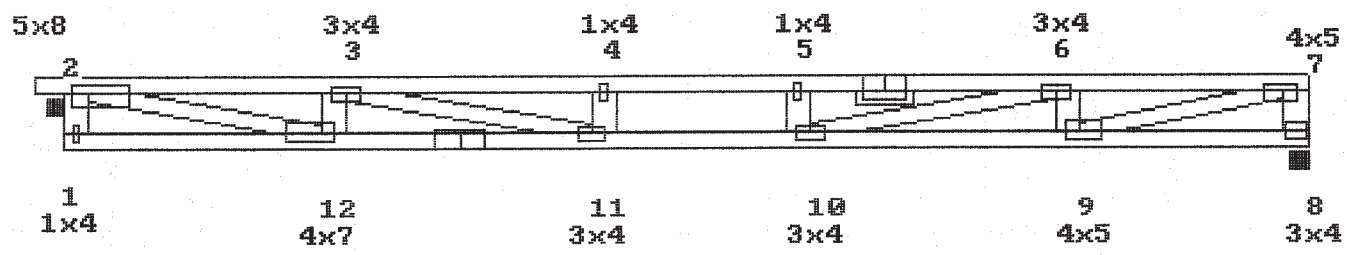
====<<<<A.C.E.S Version 7.2>>>>===== [001680] =====>>>>BEST.TRUSS>>>>====
 Hand cut>>>>: 18.69 Hand jig>>>>: 19.86 Hand assembly>>>>: 14.90
 Comp cut : 8.23 Comp assembly : 0.00
 Customer : MARK-IV Fri Mar 15 15:27:46 1996
 Project #: 5K57 Truss ID : FF2 Family #: 303
 Span : 14-8 Quantity : 4 Top Pitch : /12

REACTIONS - SIZE
 2--874 2.50
 8--801 2.50

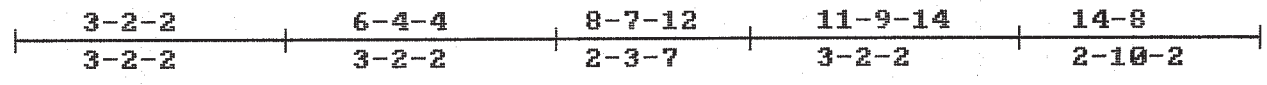
PLATE OFFSETS (X=LEFT,Y=TOP): [j2=2.5,2], [j7=3,1.5], [j9=2,2.5], [j12=5,2.5],



5x6 SPL.
 W/B



4x7 SPL.



LEFT HEIGHT:1-4 SPAN:14-8 RISE:1-4 RIGHT HEIGHT:1-4

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD:	WEBS
40	10	3-4=0.334	10-11=0.748	2*4	2*4
0	5	LL.DEFL.@10=0.16 < L/360		No.2 19 SP	No.3 19 SP

STR.INC.: LUMB = 1.00 PLATE = 1.00 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.00	1.00	2- 7= 100 8- 1= 10

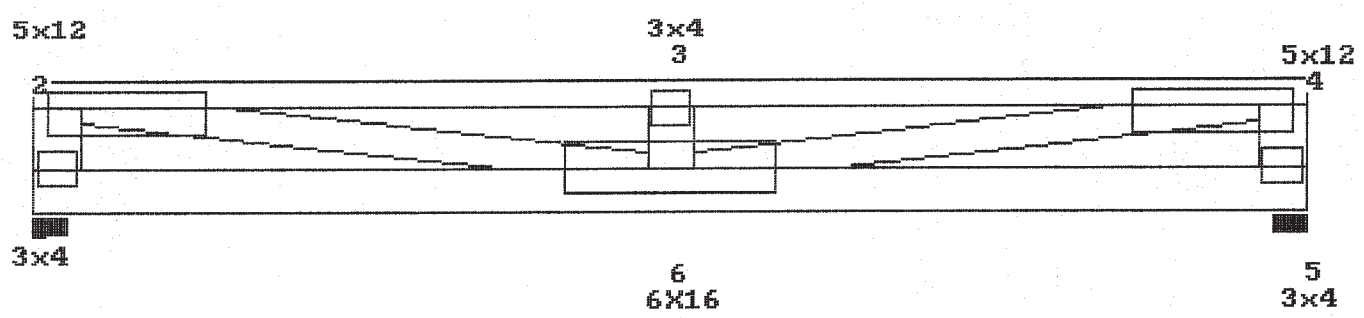
BOTH END VERTICALS ARE NOT EXPOSED.
 PLATES ARE MITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS WITH NDS DESIGN SPECS, SSBC,NDS-91
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====<<<<A.C.E.S Version 7.2>>>>===== [001681] =====<<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 5.91 Hand jig>>>>: 7.85 Hand assembly>>>>: 4.71
 Comp cut : 2.60 Comp assembly : 0.00
 Customer : MARK-IV Fri Mar 15 15:27:56 1996
 Project #: 5K57 Truss ID : FG1 Family #: 303
 Span : 8-0 Quantity : 2 Top Pitch : /12

TOP CHORD	BOTTOM CHORD	WEBS	REACTIONS -	SIZE
1-2=-1123	5-6= 0	2-6= 3887	1=-1857	2.50
2-3=-3775	6-1= 0	3-6=-390	5=-1857	2.50
3-4=-3775		4-6= 3887		
4-5=-1123				

PLATE OFFSETS (X=LEFT,Y=TOP): [j2=2.5,2], [j4=9.5,2],



LEFT HEIGHT:1-4 SPAN:8-0 RISE:1-4 RIGHT HEIGHT:1-4

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	BOTT	TOP	BOTT	TOP CHORD	BOT CHORD
40	0	2-3=0.298	5-6=0.402	2*4 No.2 19 SP	2*6 No.2 19 SP
10	5	LL.DEFL.@6=0.07 < L/360		WEBS :2*4 No.3 19 SP	

STR.INC.: LUMB = 1.00 PLATE = 1.00 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES NOT USED NO. OF MEMBERS = 2

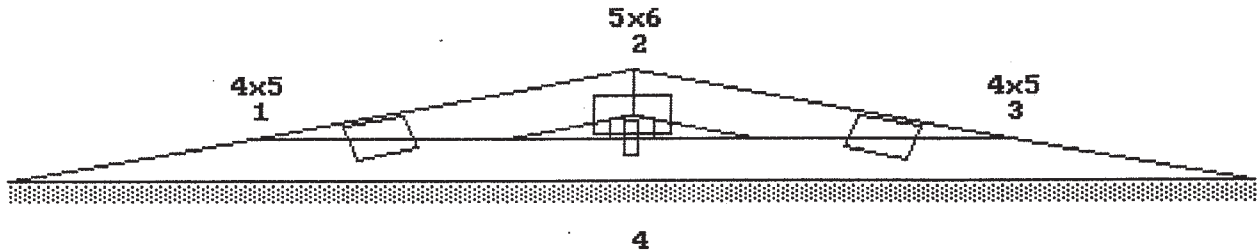
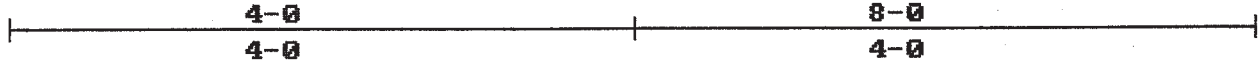
LOADING STRESS INCREASE	LOADING LUMBER	LOADING PLATE	LOADING TYPE	PANEL(PLF) / JOINTS(LBS)
1	1.00	1.00	UNIFORM	2- 4= 100 5- 1= 377

BOTH END VERTICALS ARE NOT EXPOSED.
 2 MEMBERS NAILED TOG. W/1 ROW(S) OF 16d NAILS 12 in. o.c.(TOP CHS.),AND 1 ROW(S) OF 16d NAILS 9 in. o.c.(BOTT. CHS.)
 For Webs use 1 ROW of 16d NAILS 12 in. o.c.
 PLATES ARE MITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS WITH NDS DESIGN SPECS, SSBC,TPI-91
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=====<<<<DRAWING>>>>===== [001697] =====<<<<BEST.TRUSS>>>>=====
 Hand cut>>>>: 2.65 Hand jig>>>>: 3.90 Hand assembly>>>>: 1.67
 Comp cut : 0.92 Comp assembly : 0.00
 Customer : MARK-IV Fri Mar 15 15:31:32 1996
 Project #: 5K57 Truss ID : V8-0 Family # : 310
 Span : 8-0 Quantity : 1 Top Pitch : 3.5/12

- NOTES: (1) -Gable studs spaced at 24 inches o.c.
 (2) -Brace vertical studs in accordance with standard gable end detail
 (3) -Continuous bearing provided along entire bottom chord
 (4) -Provide 1X4 plates at each end of gable stud unless otherwise noted



L. HL TO PK:4-2
 LEFT HEIGHT:0-0

SPAN:8-0

RISE:1-2

R. HL TO PK :4-2
 RIGHT HEIGHT:0-0

LOADING (PSF)		MAX STRESSES	MINIMUM GRADE OF LUMBER	
TOP	L 30 D 15	LL.DEFL.@0=0.00 < L/360	TOP CHORD:2*6	No.2 19 SP
BOTT	0 10		BOT CHORD:2*6	No.2 19 SP
			WEBS :2*4	No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED

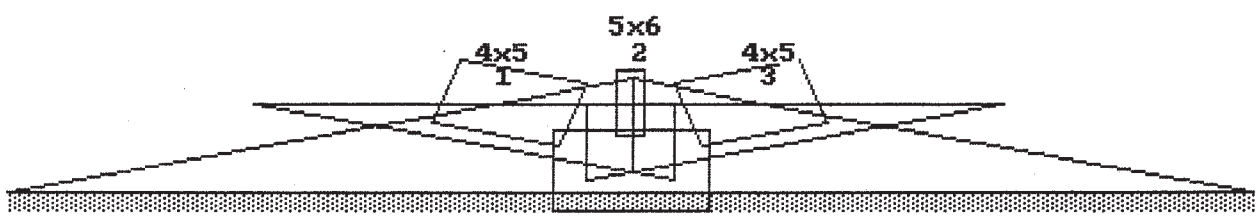
SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 10 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).
 PLATES ARE MITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS WITH NDS DESIGN SPECS, SSBC,TPI-91
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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 3/16/96

<<<<DRAWING>>>> [001696] <<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 1.57 Hand jig>>>>: 2.31 Hand assembly>>>>: 0.99
 Comp cut : 0.55 Comp assembly : 0.00
 Customer : MARK-IV Fri Mar 15 15:31:26 1996
 Project #: 5K57 Truss ID : V4-0 Family #: 310
 Span : 4-0 Quantity : 1 Top Pitch : 3.5/12

- NOTES: (1) -Gable studs spaced at 24 inches o.c.
 (2) -Brace vertical studs in accordance with standard gable end detail
 (3) -Continuous bearing provided along entire bottom chord
 (4) -Provide 1X4 plates at each end of gable stud unless otherwise noted



L. HL TO PK:2-1 R. HL TO PK :2-1
 LEFT HEIGHT:0-0 SPAN:4-0 RISE:0-7 RIGHT HEIGHT:0-0

LOADING (PSF)		MAX STRESSES	MINIMUM GRADE OF LUMBER	
L	D		TOP CHORD:2*6	No.2 19 SP
TOP	30	15	BOT CHORD:2*6	No.2 19 SP
BOTT	0	10	WEBS	:2*4 No.3 19 SP

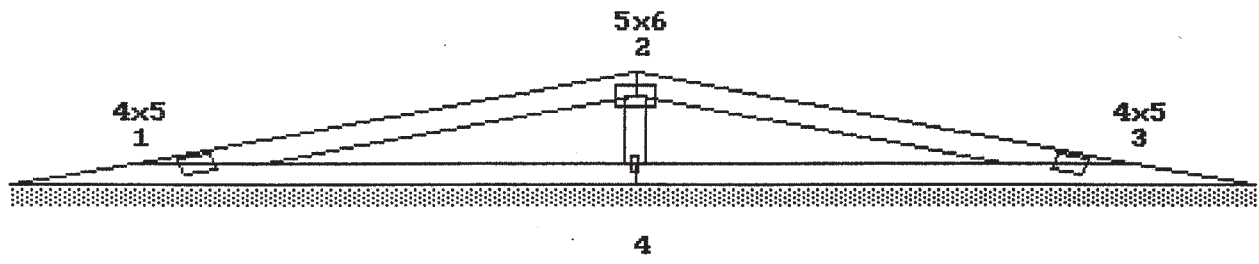
STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 10 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).
 PLATES ARE MITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS WITH NDS DESIGN SPECS, SSBC,TPI-91
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

Handwritten signature and date:
 3.6.96

====<<<<DRAWING>>>>=====[001695]====<<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 5.67 Hand jig>>>>: 8.33 Hand assembly>>>>: 3.57
 Comp cut : 1.97 Comp assembly : 0.00
 Customer : MARK-IV Fri Mar 15 15:31:19 1996
 Project #: 5K57 Truss ID : V16-0 Family #: 310
 Span : 16-0 Quantity : 1 Top Pitch : 3.5/12

- NOTES: (1)-Gable studs spaced at 24 inches o.c.
 (2)-Brace vertical studs in accordance with standard gable end detail
 (3)-Continuous bearing provided along entire bottom chord
 (4)-Provide 1X4 plates at each end of gable stud unless otherwise noted



L. HL TO PK:8-4 R. HL TO PK :8-4
 LEFT HEIGHT:0-0 SPAN:16-0 RISE:2-4 RIGHT HEIGHT:0-0

LOADING (PSF)	MAX STRESSES	MINIMUM GRADE OF LUMBER
L D		TOP CHORD:2*6 No.2 19 SP
TOP 30 15		BOT CHORD:2*6 No.2 19 SP
BOTT 0 10	LL.DEFL.@0=0.00 < L/360	WEBS :2*4 No.3 19 SP

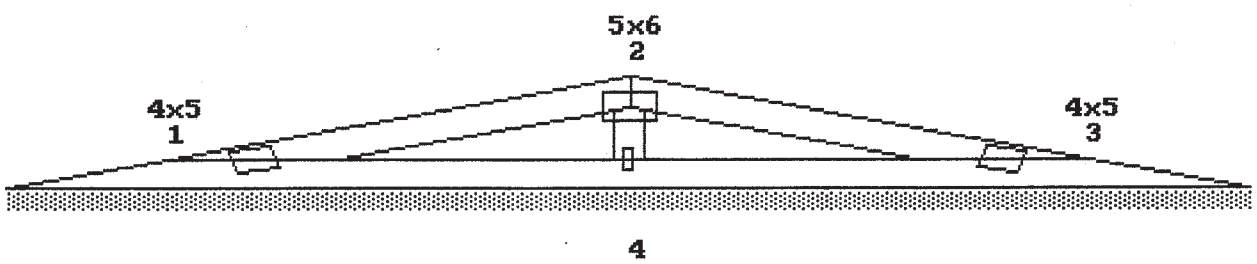
STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 10 FT,BLDG. CATG. 1,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).
 PLATES ARE MITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS WITH NDS DESIGN SPECS, SSBC,TPI-91
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 3-16-96

====<<<<DRAWING>>>====[001694]====<<<<BEST.TRUSS>>>====
 Hand cut>>>>: 4.10 Hand jig>>>>: 6.03 Hand assembly>>>>: 2.58
 Comp cut : 1.43 Comp assembly : 0.00
 Customer : MARK-IV Fri Mar 15 15:31:12 1996
 Project #: 5K57 Truss ID : V12-0 Family # : 310
 Span : 12-0 Quantity : 1 Top Pitch : 3.5/12

- NOTES: (1)-Gable studs spaced at 24 inches o.c.
 (2)-Brace vertical studs in accordance with standard gable end detail
 (3)-Continuous bearing provided along entire bottom chord
 (4)-Provide 1X4 plates at each end of gable stud unless otherwise noted



L. HL TO PK:6-3 R. HL TO PK :6-3
 LEFT HEIGHT:0-0 SPAN:12-0 RISE:1-9 RIGHT HEIGHT:0-0

LOADING (PSF)		MAX STRESSES	MINIMUM GRADE OF LUMBER	
L	D		TOP CHORD	BOT CHORD
30	15		2*6 No.2 19 SP	2*6 No.2 19 SP
0	10	LL.DEFL.@0=0.00 < L/360	WEBS :2*4	No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

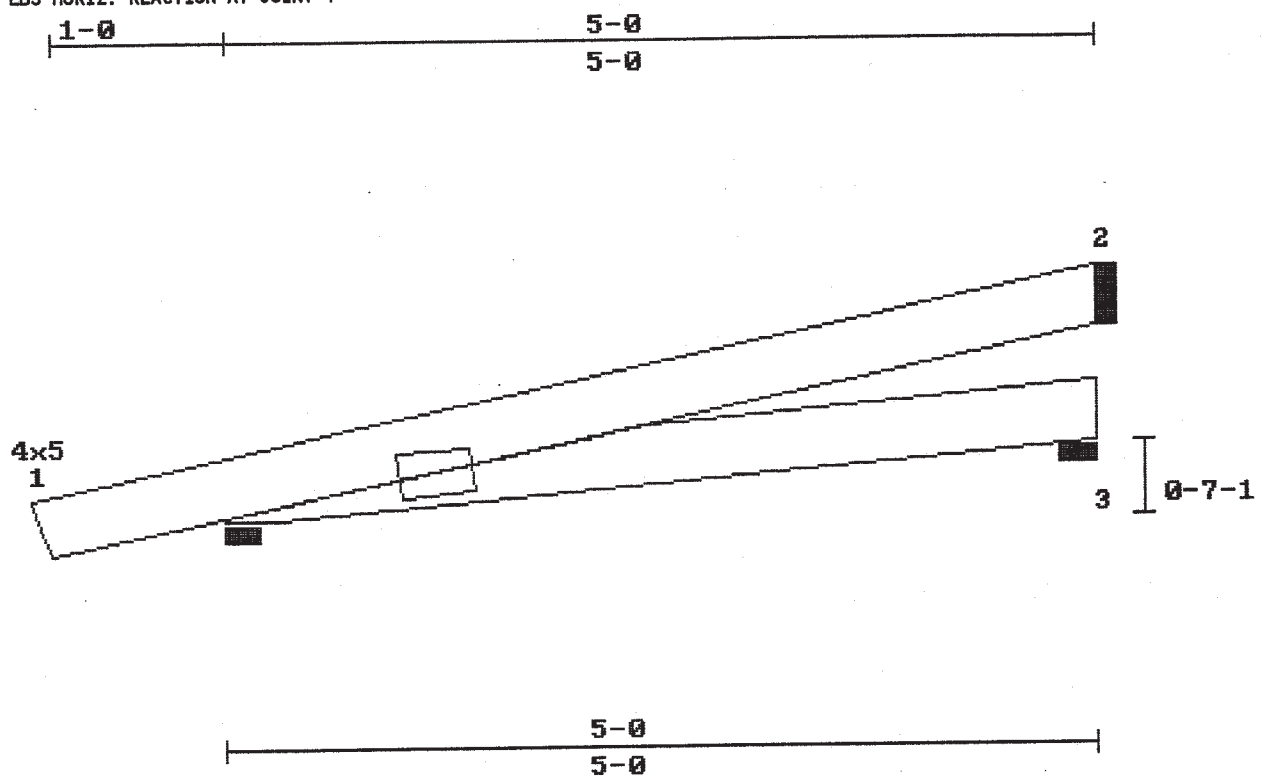
TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 10 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).
 PLATES ARE NITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS WITH NDS DESIGN SPECS, SSBC,TPI-91
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====<<<<A.C.E.S Version 7.2>>>>====[001692]====<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 3.72 Hand jig>>>>: 4.94 Hand assembly>>>>: 2.96
 Comp cut : 1.64 Comp assemlly : 0.00
 Customer : MARK-IV Fri Mar 15 15:30:47 1996
 Project #: 5K57 Truss ID : SJ5 Family # : 219
 Span : 5-0 Top Pitch : 3.5/12
 Seat cut : 0-3-8 Bot. pitch : 1.5/12

FORCES - GRAVITY LOADS REACTIONS - SIZE
 1-2= 0 3-1= 0 1=-374 2.50
 2-3= 0 3=-48 2.50
 2=-216 1.50

PROVIDE FOR 246 LBS UPLIFT AT JOINT 1 (0.66)
 PROVIDE FOR 167 LBS UPLIFT AT JOINT 2 (0.77)
 PROVIDE FOR 118 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:5-2-8 INTERNAL RISE:0-7-1
 LEFT HEIGHT:0-6 SPAN:5-0 RISE:1-11-8 RIGHT HEIGHT:1-4-7

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	L 30 D 15	TOP	1-2=0.203	TOP CHORD:2*6	No.2 19 SP
BOTT	L 0 D 10	BOTT	3-1=0.045	BOT CHORD:2*6	No.2 19 SP
		LL.DEFL.	< L/360	WEBS	:2*4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

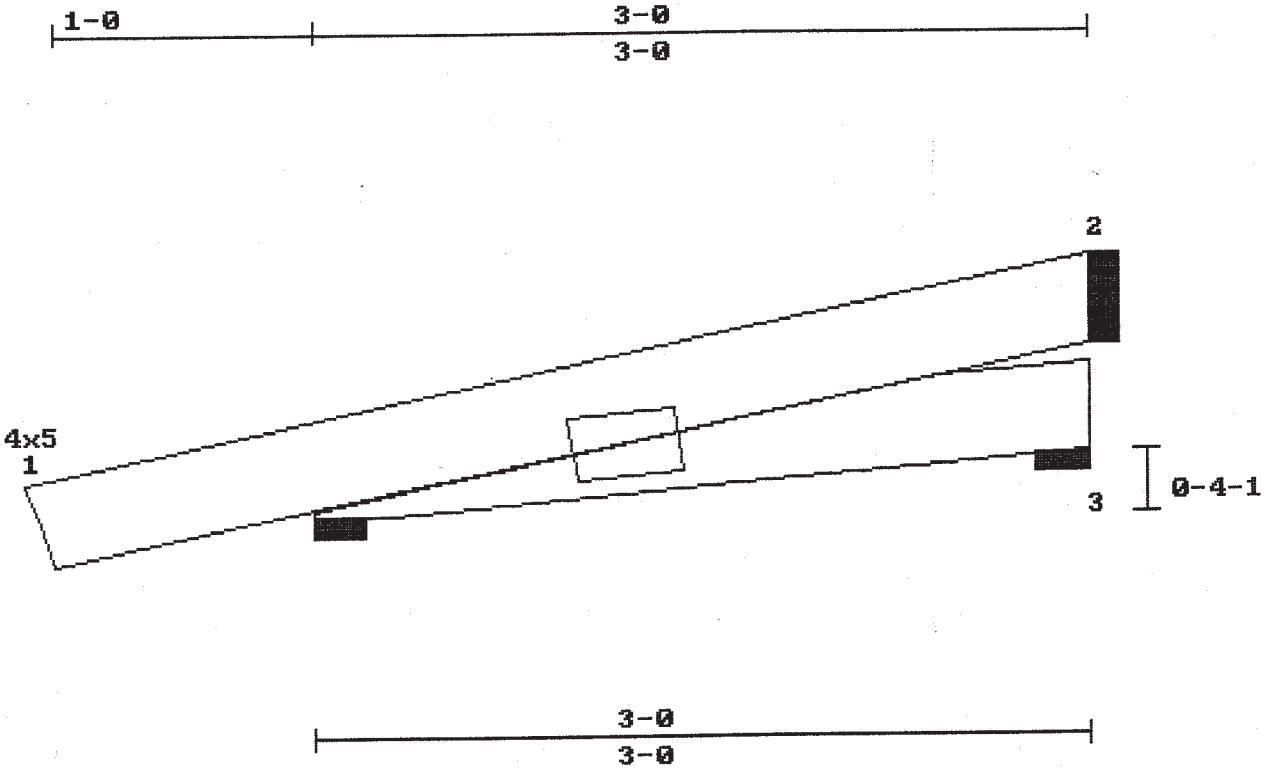
TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 10 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).
 PLATES ARE MITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS WITH NDS DESIGN SPECS, SSBC,TPI-91
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 3-16-96

====<<<<A.C.E.S Version 7.2>>>>===== [001691] =====<<<<BEST TRUSS>>>>=====
 Hand cut>>>>: 5.15 Hand jig>>>>: 5.47 Hand assembly>>>>: 4.10
 Comp cut : 2.27 Comp assembly : 0.00
 Customer : MARK-IV Fri Mar 15 15:30:33 1996
 Project #: 5K57 Truss ID : SJ3 Family # : 219
 Span : 3-0 Top Pitch : 3.5/12
 Seat cut : 0-3-8 Quantity : 4 Bot Pitch : 1.6/12

FORCES - GRAVITY LOADS REACTIONS - SIZE
 1-2= 0 3-1= 0 1=-264 2.50
 2-3= 0 3=-28 2.50
 2=-126 1.50

PROVIDE FOR 201 LBS UPLIFT AT JOINT 1 (0.76)
 PROVIDE FOR 98 LBS UPLIFT AT JOINT 2 (0.78)
 PROVIDE FOR 70 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:3-1-8 INTERNAL RISE:0-4-1
 LEFT HEIGHT:0-6 SPAN:3-0 RISE:1-4-8 RIGHT HEIGHT:1-0-7

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD:	BOT CHORD:
30	15	1-2=0.066	3-1=0.015	2*6 No.2 19 SP	2*6 No.2 19 SP
0	10	LL.DEFL. < L/360		WEBS :2*4 No.3 19 SP	

STR. INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

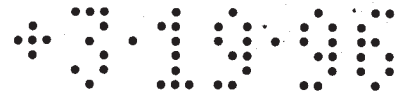
TRUSS CHECKED FOR 110 M.P.H WIND, WALL HGT. 10 FT, BLDG. CATG. I, EXP. CATG. C, 15 PSF DL(10+5), 10.00 MILES FROM OCEANLINE(ASCE7-88).
 PLATES ARE MITEK M20-258, 216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
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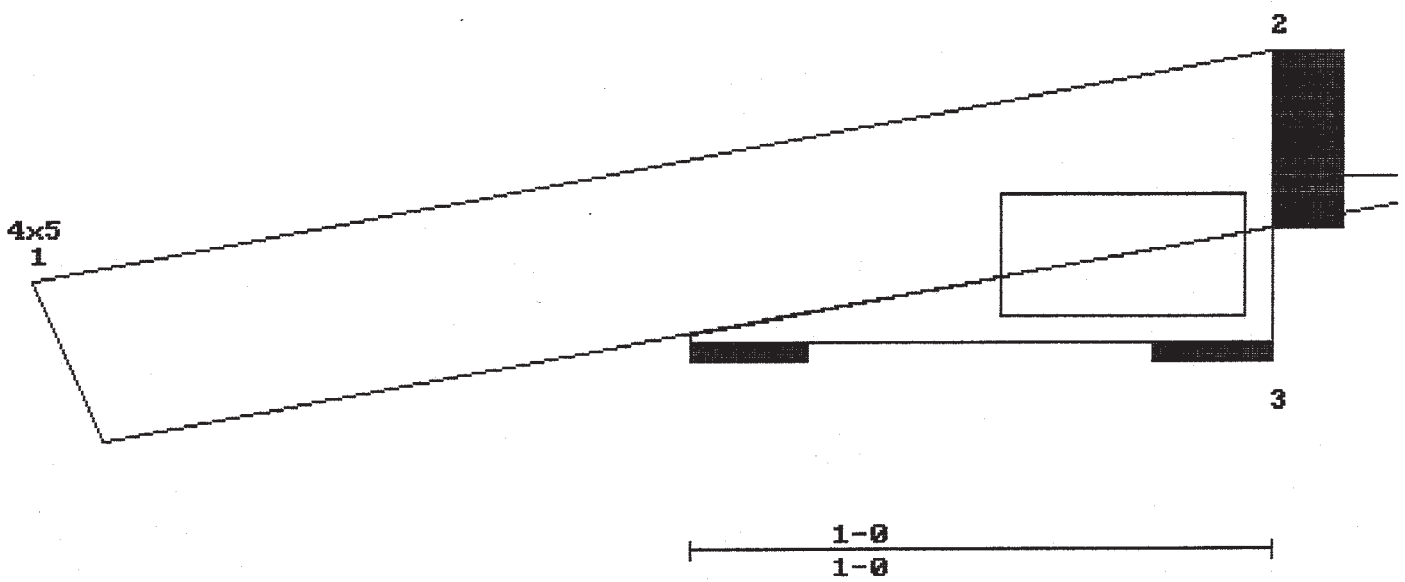
====<<<<A.C.E.S Version 7.2>>>>===== [001690] =====<<<<BEST.TRUSS>>>>=====
 Hand cut>>>>: 2.86 Hand jig>>>>: 3.04 Hand assembly>>>>: 2.28
 Comp cut : 1.26 Comp assembly : 0.00
 Customer : MARK-IV Fri Mar 15 15:30:18 1996
 Project #: 5K57 Truss ID : SJ1 Family #: 205
 Span : 1-0 Quantity : 4 Top Pitch : 3.5/12

FORCES - GRAVITY LOADS
 1-2= 0 3-1= 0
 2-3= 0

REACTIONS - SIZE
 1=-154 2.50
 3=-8 2.50
 2=-36 1.50



PROVIDE FOR 156 LBS UPLIFT AT JOINT 1 (1.02)
 PROVIDE FOR 29 LBS UPLIFT AT JOINT 2 (0.81)
 PROVIDE FOR 23 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:1-0-8
 LEFT HEIGHT:0-6 SPAN:1-0 RISE:0-9-8 RIGHT HEIGHT:0-9-8

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	30	TOP	1-2=0.004	TOP CHORD:	2*6 No.2 19 SP
BOTT	0	BOTT	3-1=0.001	BOT CHORD:	2*6 No.2 19 SP
		LL.DEFL.	< L/360	WEBS	:2*4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

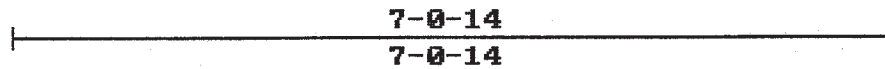
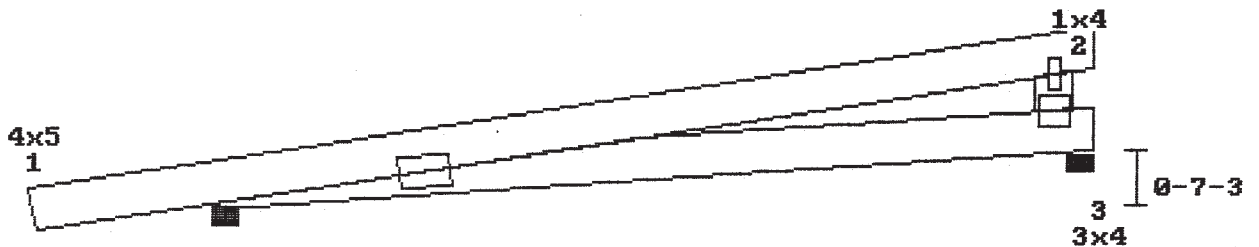
TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 10 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).
 PLATES ARE MITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS WITH NDS DESIGN SPECS. SSBC,TPI-91
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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3/16/96

====<<<<A.C.E.S Version 7.2>>>>====[001689]====<<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 5.34 Hand jig>>>>: 7.09 Hand assembly>>>>: 4.26
 Comp cut : 2.35 Comp assembly : 0.00
 Customer : MARK-IV
 Project #: 5K57 Truss ID : SCJ5
 Span : 7-0-14 Quantity : 2
 Seat cut : 0-3-8
 Bot pitch : 1.06/12
 Fri, Mar, 15 15:30:03 1996
 Family # : 215
 Top Pitch : 2.475/12

FORCES - LOAD CASE #1 REACTIONS - SIZE
 1-2= 0 3-1= 0 1=-490 2.50
 2-3=-273 3=-334 2.50

PROVIDE FOR 492 LBS UPLIFT AT JOINT 1 (1.01)
 PROVIDE FOR 321 LBS UPLIFT AT JOINT 3 (0.96)
 PROVIDE FOR 120 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:7-2-11 INTERNAL RISE:0-7-3
 LEFT HEIGHT:0-6 SPAN:7-0-14 RISE:1-11-8 RIGHT HEIGHT:1-4-5

LOADING (PSF)			MAX STRESSES		MINIMUM GRADE OF LUMBER		
TOP	L	D	TOP	1-2=0.431	TOP CHORD:	2*6	No.2 19 SP
BOTT	0	10	BOTT	3-1=0.096	BOT CHORD:	2*6	No.2 19 SP
			LL.DEFL.	< L/360	WEBS	:2*4	No.3 19 SP

REPETITIVE STRESSES NOT USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING STRESS INCREASE	LOADING LUMBER PLATE TYPE	LOADING TYPE	PANEL(PLF) / JOINTS(LBS)	
1	1.33	1.33	UNIFORM	1- 2= 80 3- 1= 18

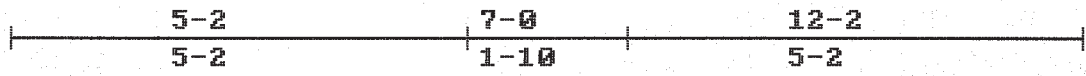
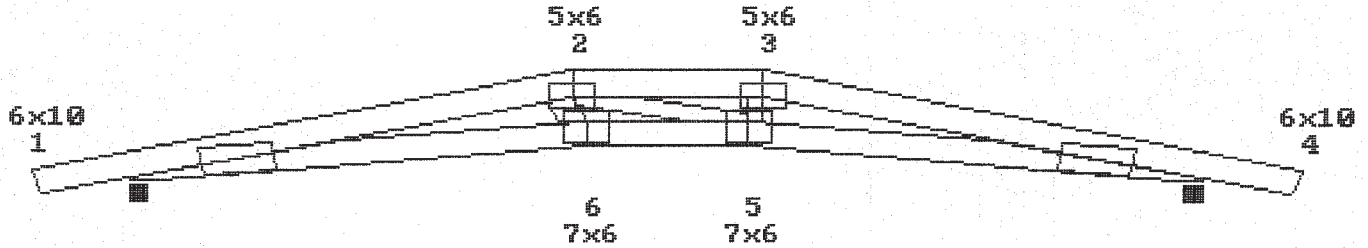
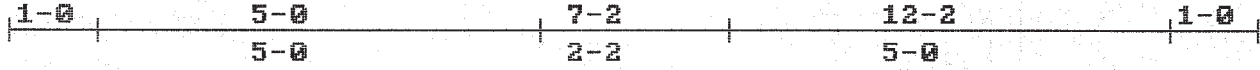
RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 10 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).
 PLATES ARE MITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS WITH NDS DESIGN SPECS, SSBC,TPI-91
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

Handwritten signature and date:
 3/16/96

====<<<<A.C.E.S Version 7.2>>>>===== [001682]=====>>>><<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 6.21 Hand jig>>>>: 9.13 Hand assembly>>>>: 3.91
 Comp cut : 2.16 Comp assembly : 0.00
 Customer : MARK-IV
 Project #: 5K57 Truss ID : HS5
 Span : 12-2 Quantity : 1
 Seat cut : 0-3-8
 Fri Mar 15 15:28:12 1996
 Family #: special
 Top Pitch : 3.5/12
 Bot. pitch : 1.5/12

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=-4939	4-5= 4757	2-6= 1071	1=-1256	2.50
2-3=-4921	5-6= 4921	2-5= 0	4=-1256	2.50
3-4=-4939	6-1= 4757	3-5= 1071		

PROVIDE FOR 599 LBS UPLIFT AT JOINT 1 (0.48)
 PROVIDE FOR 599 LBS UPLIFT AT JOINT 4 (0.48)
 PROVIDE FOR 5 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP): [j5=3,2], [j6=3,2],



L. HL TO PK:5-2-8 INTERNAL RISE:0-7-5 R. HL TO PK :5-2-8
 LEFT HEIGHT:0-6 SPAN:12-2 RISE:1-11-8 RIGHT HEIGHT:0-6

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD:	NO. 2 19 SP
30	15	1-2=0.312	4-5=0.654	2*6	No. 2 19 SP
0	10	LL.DEFL.@6=0.17 < L/360		BOT CHORD: 2*6	No. 2 19 SP
				WEBS : 2*4	No. 3 19 SP

REPETITIVE STRESSES NOT USED SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING	STRESS INCREASE	LOADING	PANEL(PLF) / JOINTS(LBS)
LUMBER	PLATE	TYPE	
1	1.33	1.33	UNIFORM 1- 2= 90 2- 3= 158 3- 4= 90 4- 5= 20 5- 6= 35 6- 1= 20
			CONCENTRATED 5= 402 6= 402

TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 10 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).
 PLATES ARE MITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
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 3/16/96

====<<<<A.C.E.S Version 7.2>>>>===== [001683] =====>>>><<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 3.00 Hand jig>>>>: 3.44 Hand assembly>>>>: 2.39
 Comp cut : 1.32 Comp assembly : 0.00
 Customer : MARK-IV
 Project #: 5K57 Truss ID : J24 Fri Mar 15 15:28:28 1996
 Span : 2-4 Quantity : 3 Family # : 205
 Top Pitch : 3.5/12

FORCES - GRAVITY LOADS

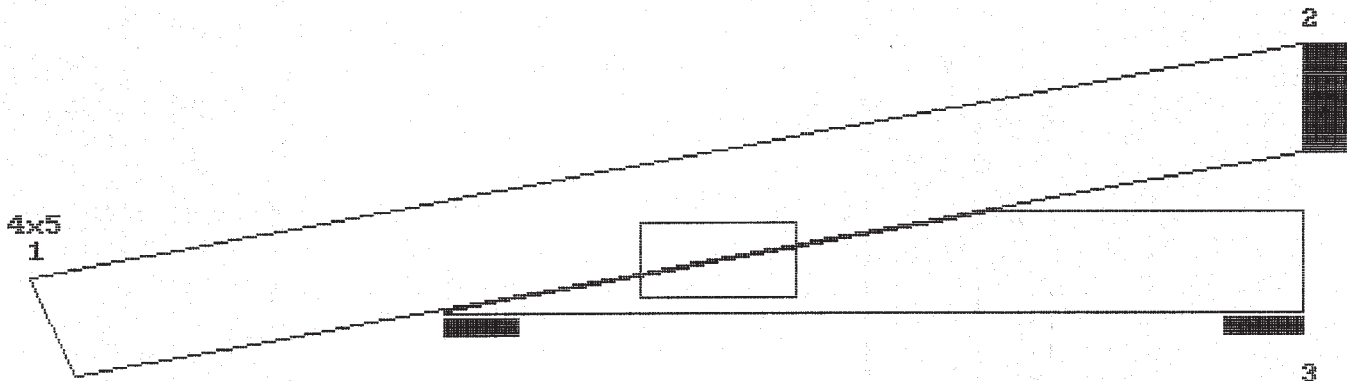
REACTIONS - SIZE

1-2= 0
2-3= 0

3-1= 0

1=-227 2.50
3=-21 2.50
2=-96 1.50

PROVIDE FOR 186 LBS UPLIFT AT JOINT 1 (0.82)
 PROVIDE FOR 75 LBS UPLIFT AT JOINT 2 (0.78)
 PROVIDE FOR 55 LBS HORIZ. REACTION AT JOINT 1



L. HL TO PK:2-5-3

LEFT HEIGHT:0-6

SPAN:2-4

RISE:1-2-3

RIGHT HEIGHT:1-2-3

LOADING (PSF)

	L	D
TOP	30	15
BOTT	0	10

MAX STRESSES

TOP 1-2=0.037
 BOTT 3-1=0.008
 LL.DEFL. < L/360

MINIMUM GRADE OF LUMBER

TOP CHORD:2*6 No.2 19 SP
 BOT CHORD:2*6 No.2 19 SP
 WEBS :2*4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 10 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).

PLATES ARE MITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)

PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS WITH NDS DESIGN SPECS, SSBC,TPI-91

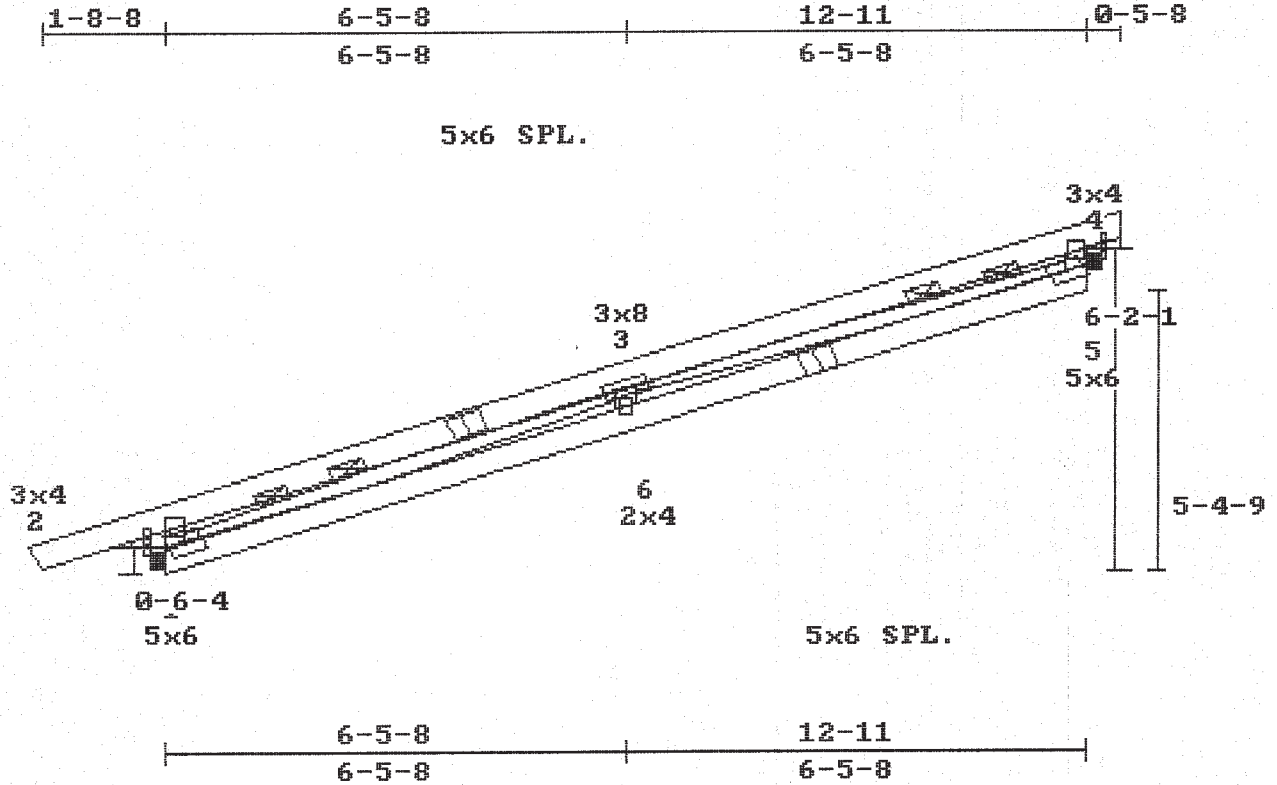
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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3-16-96

====<<<<A.C.E.S Version 7.2>>>>===== [001684] =====<<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 48.36 Hand jig>>>>: 48.75 Hand assembly>>>>: 44.46
 Comp cut : 24.57 Comp assembly : 0.00
 Customer : MARK-IV Fri Mar 15 15:28:43 1996
 Project #: 5K57 Truss ID : P1 Family #: special
 Span : 12-11 Quantity : 9 Top Pitch : 5/12
 Seat cut : 0-0 Bot pitch : 5/12

FORCES - GRAVITY LOADS			REACTIONS - SIZE	
1-2= 420	5-6= 2960	3-1=-3111	2=-898	2.50
2-3= 0	6-1= 2960	3-6= 129	4=-761	2.50
3-4= 0				
4-5= 420				

PROVIDE FOR 387 LBS UPLIFT AT JOINT 2 (0.43)
 PROVIDE FOR 381 LBS UPLIFT AT JOINT 4 (0.50)
 PROVIDE FOR 363 LBS HORIZ. REACTION AT JOINT 2



L. HL. TO PK:13-11-15

INTERNAL RISE:5-4-9

LEFT HEIGHT:1-4

SPAN:12-11

RISE:6-10-14

RIGHT HEIGHT:1-4

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD:	BOT CHORD:
30	15	2-3=0.315	6-1=0.450	2*6 No.2 19 SP	2*6 No.2 19 SP
0	10	LL.DEFL.@6=0.24 < L/360		WEBS :2*4 No.3 19 SP	

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

BOTH END VERTICALS ARE NOT EXPOSED.

WEB 3-1; 3-5 BRACED at 1/3 POINTS AS SHOWN ABOVE

Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c.

TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 11.833 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).

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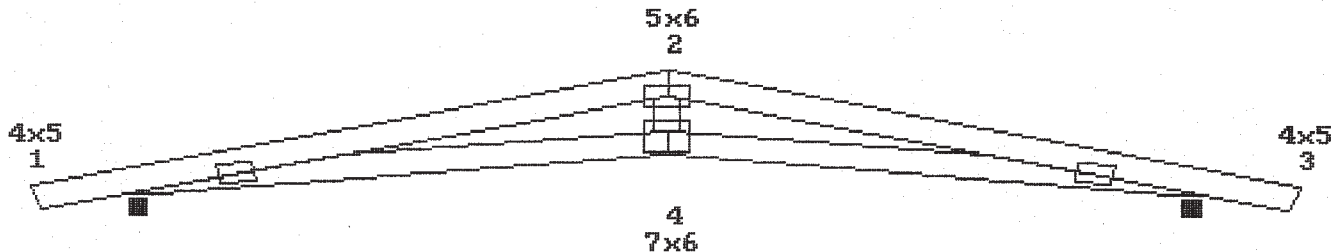
THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

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 3/16/96

====<<<<A.C.E.S Version 7.2>>>>===== [001685] =====<<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 8.77 Hand jig>>>>: 11.65 Hand assembly>>>>: 6.99
 Comp cut : 3.86 Comp assemlly : 0.00
 Customer : MARK-IV Fri Mar 15 15:29:00 1996
 Project #: 5K57 Truss ID : SC1 Family # : 115
 Span : 12-2 Quantity : 2 Top Pitch : 3.5/12
 Seat cut : 0-3-8 Bot pitch : 1.5/12
 =====

FORCES - GRAVITY LOADS REACTIONS - SIZE
 1-2=-1903 3-4= 1836 2-4= 561 1=-768 2.50
 2-3=-1903 4-1= 1836 3=-768 2.50

PROVIDE FOR 381 LBS UPLIFT AT JOINT 1 (0.50)
 PROVIDE FOR 381 LBS UPLIFT AT JOINT 3 (0.50)
 PROVIDE FOR 6 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP): [j4=3,2],



L. HL TO PK:6-4-1 INTERNAL RISE:0-8-11 R. HL TO PK :6-4-1
 LEFT HEIGHT:0-6 SPAN:12-2 RISE:2-3-4 RIGHT HEIGHT:0-6

=====
 LOADING (PSF) MAX STRESSES MINIMUM GRADE OF LUMBER
 L D TOP 1-2=0.285 TOP CHORD:2*6 No.2 19 SP
 TOP 30 15 BOTT 3-4=0.299 BOT CHORD:2*6 No.2 19 SP
 BOTT 0 10 LL.DEFL.@4=0.07 < L/360 WEBS :2*4 No.3 19 SP
 =====

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

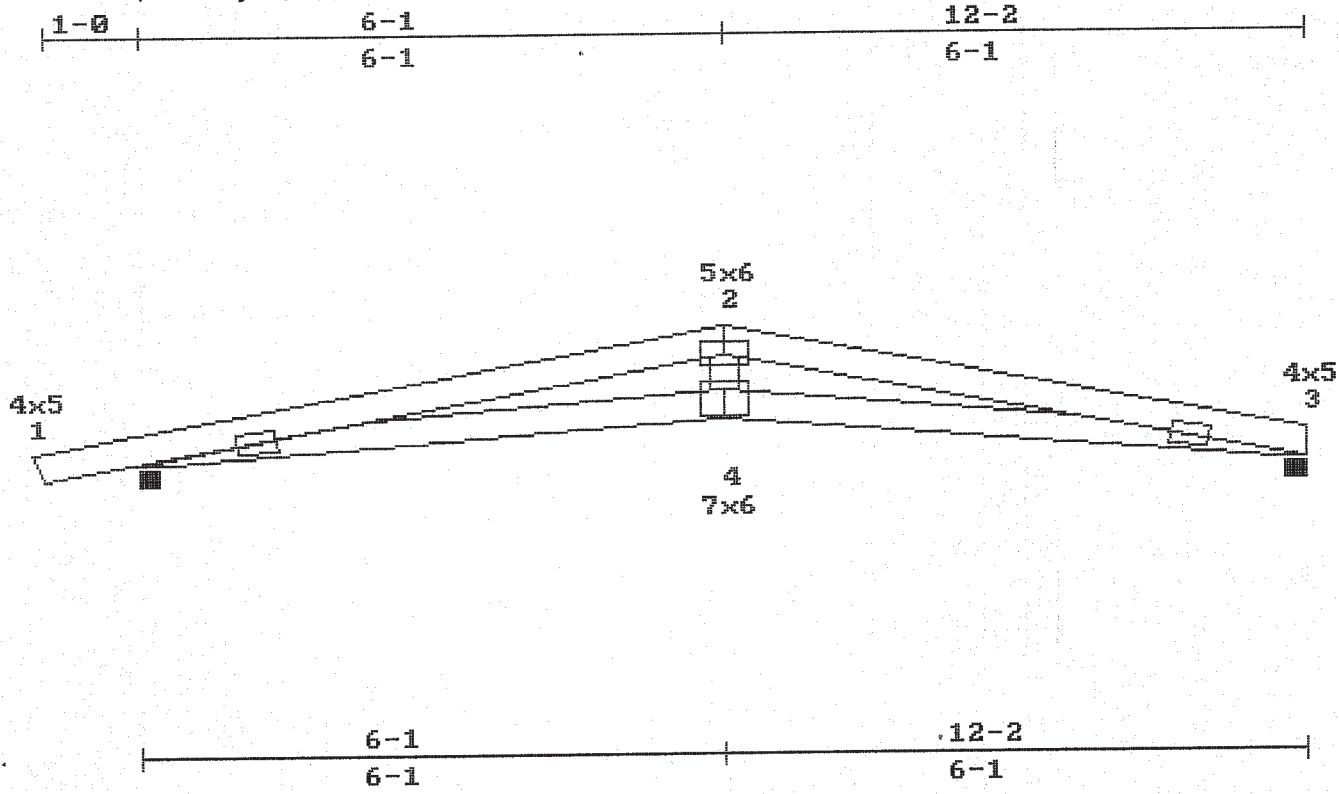
TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 10 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).
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 3-16-96

====<<<<A.C.E.S Version 7.2>>>>===== [001686] =====<<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 5.37 Hand jig>>>>: 7.89 Hand assembly>>>>: 3.38
 Comp cut : 1.87 Comp assembly : 0.00
 Customer : MARK-IV Project #: 5K57 Truss ID : SC1A Fri Mar 15 15:29:15 1996
 Span : 12-2 Quantity : 1 Family # : 115
 Seat cut : 0-3-8 Top Pitch : 3.5/12
 Bot pitch : 1.5/12

FORCES - GRAVITY LOADS REACTIONS - SIZE
 1-2=-1903 3-4= 1836 2-4= 561 1=-768 2.50
 2-3=-1903 4-1= 1836 3=-658 2.50

PROVIDE FOR 371 LBS UPLIFT AT JOINT 1 (0.48)
 PROVIDE FOR 242 LBS UPLIFT AT JOINT 3 (0.37)
 PROVIDE FOR 6 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP): [j4=3,2],



L. HL TO PK:6-4-1 INTERNAL RISE:0-8-11 R. HL TO PK :6-4-1
 LEFT HEIGHT:0-6 SPAN:12-2 RISE:2-3-4 RIGHT HEIGHT:0-6

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
TOP	L 30 D 15	TOP	1-2=0.285	TOP CHORD:	2*6 No.2 19 SP
BOTT	0 10	BOTT	3-4=0.299	BOT CHORD:	2*6 No.2 19 SP
		LL.DEFL.@4	=0.07 < L/360	WEBS	:2*4 No.3 19 SP

STR.INC.: LUMB = 1.33 PLATE = 1.33 SPACING : 24.0 in. o. c.
 REPETITIVE STRESSES USED NO. OF MEMBERS = 1

TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 10 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).
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Handwritten signature and date:
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 9-16-06

====<<<<A.C.E.S Version 7.2>>>>===== [001687] =====<<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 21.88 Hand jig>>>>: 25.09 Hand assembly>>>>: 17.44
 Comp cut : 9.64 Comp assembly : 0.00
 Customer : MARK-IV Project #: 5K57 Truss ID : SC2 Family #: special
 Span : 17-2 Quantity : 3 Top Pitch : 3.5/12
 Seat cut : 0-3-8 Bot pitch : 1.5/12

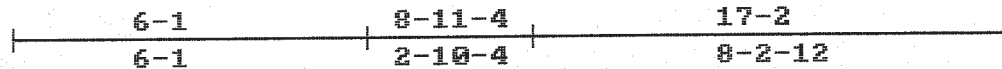
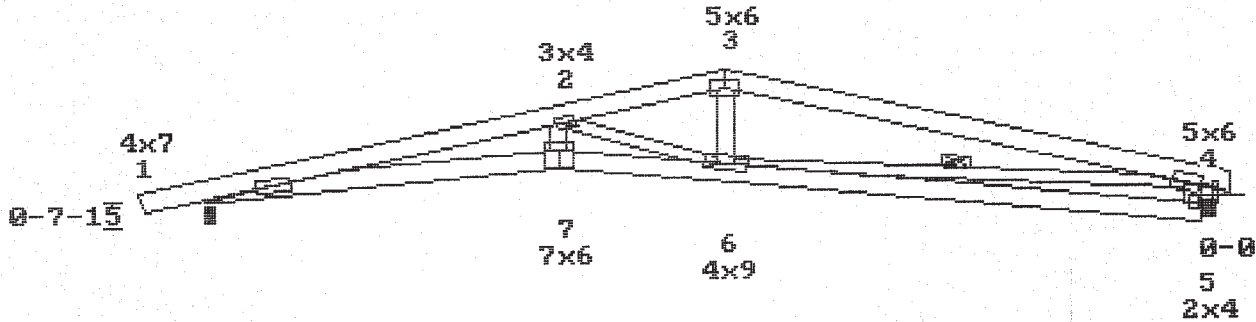
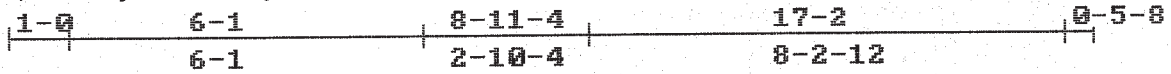
FORCES - GRAVITY LOADS

1-2=-3523 5-6= 0 2-7= 918
 2-3=-1835 6-7= 3399 2-6=-1804
 3-4=-1832 7-1= 3397 3-6= 520
 4-5= 82 4-6= 1761

REACTIONS - SIZE

1=-1048 2.50
 4=-989 2.50

PROVIDE FOR 474 LBS UPLIFT AT JOINT 1 (0.45)
 PROVIDE FOR 404 LBS UPLIFT AT JOINT 4 (0.41)
 PROVIDE FOR 28 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP): [j6=4.5,2.5], [j7=3,2],



L. HL TO PK:9-3-11
 LEFT HEIGHT:0-6

INTERNAL RISE:1-4-10
 SPAN:17-2 RISE:3-9-3

R. HL TO PK :8-6-14
 RIGHT HEIGHT:1-4-7

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD:	BOT CHORD:
30	15	3-4=0.702	6-7=0.532	2*6 No.2 19 SP	2*6 No.2 19 SP
0	10	LL.DEFL.@7=0.14 < L/360		WEBS :2*4 No.3 19 SP	

STR.INC.: LUMB = 1.33 PLATE = 1.33
 REPETITIVE STRESSES USED

SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

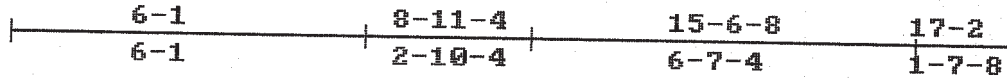
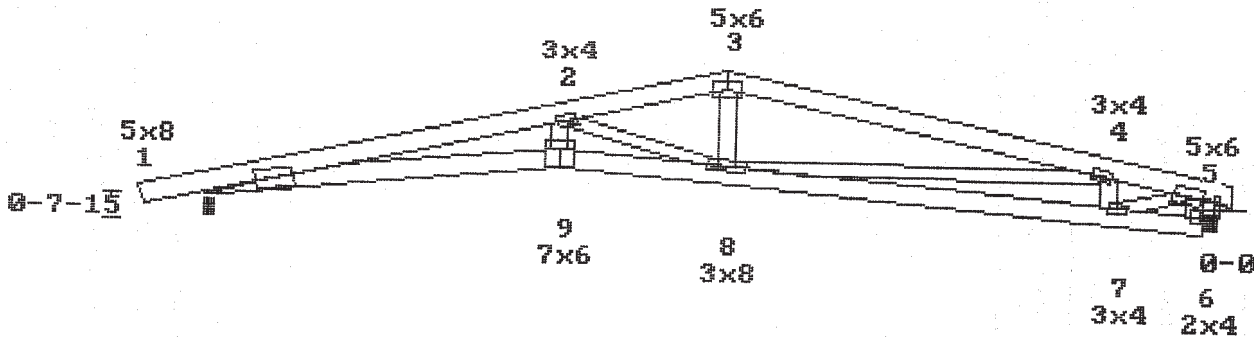
RIGHT END VERTICAL IS NOT EXPOSED.
 WEB 4-6 BRACED at 1/2 POINTS AS SHOWN ABOVE
 Note:Use 1x4 or 2x3 Cont.Bracing conn.w/2-8d nails,or T-brace of same size and grade as web conn. to narrow face w/10d nails 6 in. o.c.
 TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 10 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).
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mm
 3-6-96

====<<<<A.C.E.S Version 7.2>>>>=====[001688]=====<<<<BEST.TRUSS>>>>====
 Hand cut>>>>: 9.47 Hand jig>>>>: 13.92 Hand assembly>>>>: 5.97
 Comp cut : 3.30 Comp assemlly : 0.00
 Customer : MARK-IV Fri Mar 15 15:29:48 1996
 Project #: 5K57 Truss ID : SC2A Family #: special
 Span : 17-2 Quantity : 1 Top Pitch : 3.5/12
 Seat cut : 0-3-8 Bot pitch : 1.5/12

FORCES - LOAD CASE #1			REACTIONS - SIZE	
1-2=-4269	6-7= 0	2-9= 1094	1=-1177	2.50
2-3=-2358	7-8= 1700	2-8=-2041	5=-1354	2.50
3-4=-2358	8-9= 4118	3-8= 895		
4-5=-1745	9-1= 4116	4-8= 577		
5-6= 16		4-7=-613		
		5-7= 1853		

PROVIDE FOR 526 LBS UPLIFT AT JOINT 1 (0.45)
 PROVIDE FOR 550 LBS UPLIFT AT JOINT 5 (0.41)
 PROVIDE FOR 28 LBS HORIZ. REACTION AT JOINT 1
 PLATE OFFSETS (X=LEFT,Y=TOP): [j9=3,2],



L. HL TO PK: 9-3-11 INTERNAL RISE: 1-4-10 R. HL TO PK : 8-6-14
 LEFT HEIGHT: 0-6 SPAN: 17-2 RISE: 3-9-3 RIGHT HEIGHT: 1-4-7

LOADING (PSF)		MAX STRESSES		MINIMUM GRADE OF LUMBER	
L	D	TOP	BOTT	TOP CHORD	BOT CHORD
30	15	3-4=0.364	8-9=0.602	2*6 No.2 19 SP	2*6 No.2 19 SP
0	10	LL.DEFL.@9=0.16 < L/360		WEBS :2*4 No.3 19 SP	

REPETITIVE STRESSES NOT USED SPACING : 24.0 in. o. c.
 NO. OF MEMBERS = 1

LOADING STRESS INCREASE	LOADING TYPE	PANEL(PLF) / JOINTS(LBS)
LUMBER 1.33	PLATE 1.33	UNIFORM 1- 5= 90 6- 7= 20 7- 8= 84 8- 1= 20
		CONCENTRATED 7= 70

RIGHT END VERTICAL IS NOT EXPOSED.
 TRUSS CHECKED FOR 110 M.P.H WIND,WALL HGT. 10 FT,BLDG. CATG. I,EXP. CATG. C,15 PSF DL(10+5),10.00 MILES FROM OCEANLINE(ASCE7-88).
 PLATES ARE MITEK M20-258,216 MANUFACTURED FROM ASTM A 446 GRD A GALVANIZED STEEL(EXCEPT AS SHOWN)
 PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY(EXCEPT AS SHOWN)DESIGN CONFORMS WITH NDS DESIGN SPECS, SSBC,TPI-91
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY.FOR PERMANENT AND TEMPORARY BRACING(WHICH IS ALWAYS REQD)CONSULT BLDG ARCHITECT OR ENGINEER.

mt
 3/16/96

Permit 95090385

App 94120686

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